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### SECTION I

#### Study Introduction

Coler & Colantonio, Inc. has analyzed traffic impacts associated with the proposed Cedar Ridge Estates in the Town of Holliston, MA. This introductory section briefly describes the project site location, development program, the study area, and the traffic analysis methodology.

#### **Project Site**

The proposed site for the Cedar Ridge Estates is a parcel of land located on the southwest corner of the intersection of Marshall Street and Prentice Street in Holliston, Massachusetts. The location of the site is indicated in Figure 1. The site is bordered to the northeast by Marshall Street, to the southeast by abutting properties, to the north by Prentice Street, to the west by the Hopkinton/Holliston town line, and to the south by residential properties and undeveloped land. The surrounding land use is primarily residential or undeveloped. The existing site is currently wooded and undeveloped. There are two houses with frontage along Marshall Street located just off the southeast border of the site.

#### **Development Program**

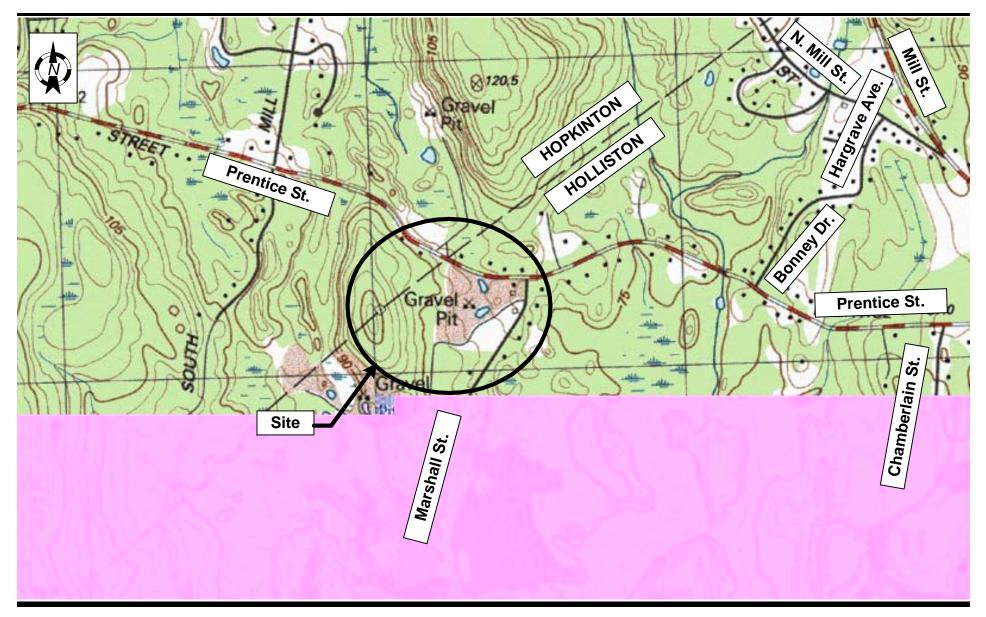
The proposed Cedar Ridge Estates project includes the development of 200 residential townhouse style condominiums. The units will have access provided by two site driveways on Marshall Street just south of Prentice Street. Both site driveways will provide access to all of the units within the site. An emergency only access to the site located at the southern end of the site is also proposed. Within the site, all of the units will have individual driveways.

#### Study Area

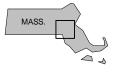
The study area for the residential condominium project was determined by looking at the intersections in the vicinity of the site that may see the most change in traffic volumes due to the proposed project. The following intersections were studied:

- Marshall Street @ Prentice Street
- Marshall Street @ Proposed Site Driveways

In addition to the study area intersections listed above, and evaluation of the full length of Marshall Street was also completed.







**Project Locus Map** 

Cedar Ridge Estates Holliston, Massachusetts

Figure 1

#### Traffic Analysis Methodology

Traffic volume data was collected for the study area intersections and roadways. These volumes were used to evaluate the existing operation of the above study area intersections in terms of capacity, delay experienced by motorists, and level of service. Future operation without the project was evaluated by projecting the existing traffic volumes to future levels based on historic traffic growth and any specific developments planned for the area. Future operations for the full-build project were evaluated by adding project-related traffic to the projected future no-build traffic volumes. Traffic impacts associated with the proposed project were determined by comparing existing and future operations without the project to the future operations with the project. Additionally, accident statistics and sight distance measurements were collected for the study area roadways and intersections. This information was used to evaluate the operational safety associated with the study area intersections.

# **SECTION II Existing Conditions**

The evaluation of existing conditions includes a description of geometry, traffic control, and land uses in the vicinity of the study area roadways and intersections; quantification of existing daily and peak hour traffic volumes; and, a review of accident data within the study area

#### Inventory of Study Area Roadways & Intersections

#### Roadways

Prentice Street

Prentice Street in Holliston is an east-west urban minor arterial, which runs from the Holliston/Hopkinton town line to Highland Street. Prentice Street is a town accepted and maintained roadway.

In the vicinity of the site, Prentice Street is a two-lane roadway with varying width. West of the intersection with Marshall Street, Prentice Street is 20 feet wide. East of the intersection with Marshall Street, Prentice Street is 28 feet wide. The roadway is striped with a double yellow centerline for approximately 10-foot travel lanes west of the intersection with Marshall Street, and 14-foot travel lanes east of the intersection with Marshall Street. There is no sidewalk along the roadway. The posted speed limit on Prentice Street is 35 mph within the study area. The land use along Prentice Street is primarily residential or undeveloped.

#### Marshall Street

Marshall Street in Holliston is a north-south urban extension, which runs from Prentice Street to Adams Street. Marshall Street is a town accepted and maintained roadway. Marshall Street holds a "scenic road" designation in the Town of Holliston.

Marshall Street is a two-lane roadway with varying width and no posted speed limit. From Prentice Street to a point 2000 feet south of the proposed emergency access driveway, Marshall Street has a varying width of 21 to 22 feet, has no curbing, no berm, and no pavement markings. Roadside obstructions along this section of Marshall Street include trees and utility poles within 1 foot of the roadway, and vertical and horizontal curves that limit sight distance. From 2000 feet south of the proposed emergency access driveway to the end of Marshall Street at the intersection with Adams Street, Marshall Street has a varying width of 18 to 20 feet, has no curbing, alternates between having berm and not, and has no striping except at the intersection with Courtland Street. Roadside obstructions along this section of Marshall Street include trees and utility poles within 1 foot of the roadway and stonewall within 2 feet of the roadway. There are horizontal curves and vertical curves that limit sight distance within this section of Marshall Street. There are no sidewalks on any part of Marshall Street. A table including the information found during the Marshall Street roadway inventory is included in the Appendix.

The Holliston Youth Soccer Association (HYSA) has recently constructed a soccer field complex on the east side of Marshall Street, across from the approximate location of the

proposed emergency access driveway. The complex includes two full use soccer fields, four practice fields, and 230 parking spaces. The HYSA has games scheduled on Saturdays from 9am-3pm.

Two notable intersections exist along Marshall Street. Courtland Street intersects Marshall Street as a "T" intersection approximately 1.1 miles south of the proposed emergency access driveway. Courtland Street is under stop sign control. An island exists at the intersection, allowing for two way traffic flow on each side of the island. On Courtland Street, a double yellow centerline on each side of the island designates lanes for entering and exiting. Marshall Street is 22 feet wide approaching the intersection from the north, and 27 feet wide approaching the intersection is 21 feet wide. The speed limit on Marshall Street approaching the intersection is 20 mph. Marshall Street, south of the intersection begins to proceed in a westerly direction.

Marshall Street terminates at its intersection with Adams Street as a "T" intersection. At the intersection, Marshall Street is under stop control, is 19 feet wide, and has a bituminous berm along both sides of the roadway. The speed limit along Adams Street is 35 mph north of the intersection, and 30 mph south of the intersection.

#### Intersections

Marshall Street @ Prentice Street

Marshall Street intersects Prentice Street to form a three-legged "T" intersection with Marshall Street under stop conditions. Prentice Street, at this intersection, is a two-lane roadway with varying width. West of the intersection with Marshall Street, Prentice Street is 20 feet wide. East of the intersection with Marshall Street, Prentice Street is 28 feet wide. The roadway is striped with a double yellow centerline for approximately 10-foot travel lanes west of the intersection with Marshall Street, and 14-foot travel lanes east of the intersection with Marshall Street. No curbing, sidewalks, or striping exist along the intersection. There is a stop sign located at the intersection for vehicles along Marshall Street approaching the intersection.

The land uses of properties around the Marshall Street at Prentice Street intersection include a single house on the immediate southwest corner of the intersection, houses along the north side of Prentice Street, and a wooded area on the southeast corner.

#### Existing Traffic Volumes

In order to determine base traffic volume conditions, manual turning movement counts and automatic traffic recorder counts (ATR) were performed. Manual turning movement counts were performed May 1<sup>st</sup> to May 3<sup>rd</sup> 2003. ATR counts were originally performed in May of 2003 to determine the daily traffic on Marshall Street and Prentice Street. Subsequent ATR counts were performed on Prentice Street at the location of the proposed site on Tuesday September 20<sup>th</sup> 2005 and again from Friday October 14<sup>th</sup> to Sunday October 16<sup>th</sup> 2005 to determine if traffic volumes had increased since the original counts were performed.

Manual turning movements counts were performed during the weekday morning peak period (7:00 am to 9:00 am) and weekday evening peak period (4:00 to 6:00 pm) at the study area

intersection. The actual peak hours for traffic volumes in the study area were found to be 7:00 to 8:00 am and 4:45 to 5:45 pm. Figure 2 presents the 2005 existing weekday morning and weekday evening peak hour traffic volume conditions. Copies of the actual traffic volume data are provided in the Appendix.

To determine if the measured turning movement count volumes would need to be adjusted to reflect a average monthly volumes, traffic volume data from the Massachusetts Highway Department (MHD) 2003 Weekday Seasonal Factors was used. A copy of this table is included in the Appendix. The table revealed that monthly traffic conditions in the month of May are historically around average monthly traffic volume conditions. Therefore, the May traffic volumes were not adjusted, and the analyses presented in this report represent an average month condition. However, since the turning movement counts were performed in May of 2003, the counts were adjusted forward by a percentage to meet September 2005 conditions. The percentage used was based on the average rate of increase from the ATR counts for Prentice Street and Marshall Street from 2003 to 2005, and used to determine the adjusted volumes for the turning movement counts at the intersection of Marshall Street at Prentice Street.

ATR counts were conducted for a 24-hour period at the location of the site on Prentice Street and Marshall Street. Table 1 tabulates the average daily traffic volumes for the ATR location. Copies of the actual traffic volume data are provided in the Appendix.

**Table 1. Existing Traffic Volume Summary** 

		Average	Daily Traffic		Peak Ho		
Location	Day of Week	Volumes (vpd) <sup>a</sup>	Directional Distribution	Period	Volumes (vph) <sup>b</sup>	Directional Distribution	K factor <sup>c</sup>
Prentice Street	Weekday	4206	52% EB	Morning	441	54% EB	0.10
				Evening	431	51% EB	0.10
Marshall Street	Weekday	1347	52% NB	Morning	133	74% NB	0.10
				Evening	166	59% SB	0.12
Marshall Street	Weekend	1555	54% NB	Mid-day	209	59% NB	0.13

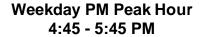
<sup>&</sup>lt;sup>a</sup> Vehicles per day

To determine if the measured ATR count volumes would need to be adjusted to reflect average monthly volumes, traffic volume data from the MHD 2003 Weekday Seasonal Factors was once again used. The table revealed that monthly traffic conditions in the month of September are historically around average monthly traffic volume conditions. Therefore, the September traffic volumes were not adjusted, and the analyses presented in this report represent an average month condition.

<sup>&</sup>lt;sup>b</sup> Vehicles per hour

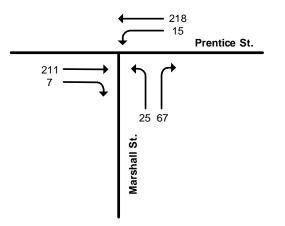
<sup>&</sup>lt;sup>c</sup> Percent of daily traffic

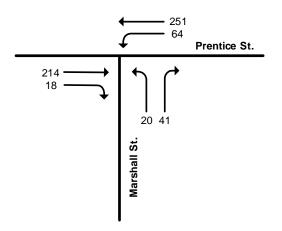


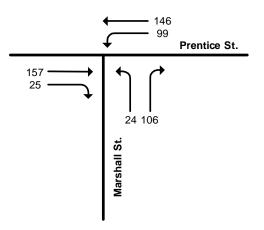


Saturday Peak Hour 11:00 AM - 12:00 PM





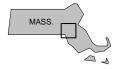




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ENGINEERS AND SCIENTISTS



2005 Existing Conditions
Traffic Volumes

Cedar Ridge Estates Holliston, Massachusetts

#### **Truck Percentages**

As part of the evaluation of existing traffic conditions the percentage of heavy vehicles (trucks and buses) was reviewed. Heavy vehicle percentages were determined using the daily traffic counts that were performed on October 14-16, 2005, which were set up to also perform vehicle classification. According to the counts, traffic on Marshall Street experienced 6.2% heavy vehicles on Friday, October 14; 3.3% heavy vehicles on Saturday, October 15; and 2.5% heavy vehicles on Sunday, October 16. Vehicles classification counts are included in the Appendix.

#### **Operational Analysis Methodology**

The capacity of an intersection or any roadway is the maximum number of vehicles that can reasonably be expected to traverse a roadway segment and/or intersection approach during a specific time period, given the physical and operational characteristics of the facility. This capacity is determined by applying adjustment factors to an ideal saturation flow rate. The level of service (LOS), an expression of the quality of driving condition, is designated in a range from "A", which provides free flow and no traffic delays, to "F", which involves vehicle backups and traffic jam conditions. Capacity (LOS "E") represents a condition of maximum possible flow, and is controlled by the alignment and cross-section design features of an intersection or roadway segment.

Capacity analyses for unsignalized intersections are based on the average control delay for each of the intersection critical movements. Control delay includes initial deceleration delay, queue move up time, stopped delay, and final acceleration delay. The estimated control delay of a movement is calculated based on the number of the available critical gaps or spacing between vehicles in the conflicting traffic stream. Table 2 summarizes the relationship between average control delay, level of service, and expected traffic delay at unsignalized intersections.

Table 2. Level of Service Criteria – Unsignalized Intersections

Available Control Delay (sec/veh)	Level of Service	Expected Delay to Critical Movements
≤ 10	A	Little or no delay
>10 and ≤15	В	Short traffic delay
>15 and ≤25	С	Average traffic delay
>25 and ≤35	D	Long traffic delay
>35 and ≤50	E	Very long traffic delay
>50	F	Severe congestion

Source: Highway Capacity Manual, Transportation Research Board (TRB), National Research Council, 2000.

#### Assessment of Existing Conditions

Capacity analyses were conducted for the 2005 existing weekday morning, weekday evening, and weekend mid-day peak hour traffic volume conditions for study area intersections. The intersections were analyzed using the Highway Capacity Software (HCS) based on the Highway Capacity Manual (HCM) methods. The results of the intersection capacity analyses are summarized in Table 3. The actual intersection capacity analyses are presented in the Appendix.

From Table 3 it is evident that all of the movements within the study area intersection are currently operating at acceptable levels of service during the weekday morning, weekday evening, and weekend mid-day peak hour periods. All of the intersection approaches operate at LOS A or B in the peak hour periods.

Table 3. 2005 Existing Conditions - Intersection Level of Service Summary

Table 3. 2003 Existing Conditions – Intersection Level of Service Summary							
	Wee	Weekday Week		kday	Saturday Mid-day		
		ng Peak our	Evening Peak Hour		Peak 1	Peak Hour	
Location/Movement	Delay (spv) <sup>a</sup>	$LOS^b$	Delay (spv)	LOS	Delay (spv)	LOS	
Marshall Street @ Prentice Street							
Prentice St. WB - LT	7.8	A	8.0	A	7.9	A	
Marshall St. NB - LR	11.8	В	12.0	В	11.4	В	

<sup>&</sup>lt;sup>a</sup> Average delay for specific intersection movement expressed in seconds per vehicle

#### Accident Data

Accident data for the study area was obtained from the Massachusetts Highway Department (MHD) for the three-year period between 2001-2003. The accident data was reviewed and accident trends at intersections within the study area were identified. Accident data provided by the Town of Holliston Police Department was used to supplement the data from the Massachusetts Highway Department. However, the Town of Holliston Police Department's data turned up no accidents at the study area intersection. The accident record analyses for each of the study area intersections are summarized in Table 4.

Crash rates for each intersection are included in Table 4. Accident crash rates are expressed in accidents per million vehicles entering the intersection and are recognized as an effective tool to measure the safety of intersections. According to the most recent MHD data, the average state crash rate unsignalized intersections is 0.80 per million entering vehicles (MEV) in MHD District 3 and 0.66 MEV statewide. In addition to the crash rates, the crash type, severity, conditions, and time of day for the accidents for each intersection are included in Table 4 based on data from MHD accident records. No fatalities were reported at the study area intersections during the three-year period researched.

Based on the data collected for each intersection, the crash rates for the study area intersection is below the state and district averages. The crash rate worksheet for each intersection is included in the Appendix. The following is a summary of the accident data for each intersection.

<sup>&</sup>lt;sup>b</sup> Level of Service

A total of 2 accidents were reported at the intersection of Marshall Street at Prentice Street yielding a crash rate of 0.33 MEV. This is well below the District 3 Average Crash Rate of 0.80 MEV for unsignalized intersections. One of the reported crash types was an angle collision and one was a rear end collision. One of the accidents reported wet roadway conditions, while one reported dry conditions. None of the accidents occurred during the weekday morning or evening peak periods. Based on the data, the most probable causes for the accidents at this location is driver error.

Table 4. Accident Summary - 2001 to 2003

Table 4. Accident cam	Marshall St. /
	Prentice St.
Year	
2001	2
2002	0
2003	0
Total	2
Crash Rate (MEV)	0.33
Туре	
Angle	1
Head-on	0
Rear-end	1
Unknown	0
Total	2
Severity	
Property Damage	2
Personal Injury	0
Fatality	0
Unknown	0
Conditions	
Dry	1
Wet	1
Ice/snow	0
Other	0
Unknown	0
Time of Day	
7:00 – 9:00 AM	0
4:00 – 6:00 PM	0
Remainder of day	2

Source: Massachusetts Highway Department.

### SECTION III

#### **Future No-Build Conditions**

To evaluate the impact of site-generated traffic volumes on the study area roadway network, trip generated traffic from the proposed development is calculated, distributed into the roadway network and assessed under year 2010 future traffic conditions. Thus, it is necessary to project future traffic conditions without the project and conduct a similar evaluation of operational conditions for the future study year, to establish a base condition to review the effect of added traffic on the existing roadway system. To determine the future 2010 condition, the following steps are included:

- 1. Existing 2005 traffic volumes are projected to 2010 using an annual background traffic growth factor;
- 2. Traffic volumes associated with any planned developments that may impact the study area are added:
- 3. Any planned improvements of the roadway network are included in the analysis; and,
- 4. Each study area location is analyzed to determine future operational statistics.

#### **Background Traffic Growth**

Traffic growth on area roadways is a direct function of the expected land development within the study area and the surrounding region. Usually, a combination of techniques is employed to estimate this growth. To account for normal background traffic growth of the surrounding region, an annual background growth rate is applied to the study area traffic volumes. This is generally supplemented by the addition of traffic impacting the study area from specifically known development projects.

#### Known Development Projects

Discussions with the City of Holliston Planning Department and a site visit inspection indicated that there is one recent development project that will increase traffic around the study area. The project includes the addition of two full-size soccer fields, four practice fields, and a parking lot located along the eastern side of Marshall Street, south of the proposed development. At the time the 2005 traffic counts were performed, the full size fields were in operation but the practice fields were still under construction.

### Average Annual Traffic Growth

To determine the average annual growth factor, the Town of Holliston census data along with the ATR counts from 2003 and 2005, were examined. U.S. Census information for the Town of Holliston indicates that the population of Holliston increased at an average rate of 0.7% per year from 1990 to 2000. It was also found that the ATR counts for Prentice Street and Marshall Street increased at a rate of 1.4% and 18.4%, respectively, over the two-year period from 2003 to 2005. As a result, a conservative annual growth rate of 3.5% was chosen for analyses for this project.

#### **Planned Improvements**

Discussions with the Town of Holliston indicated that there are no known transportation improvement projects planned that will effect traffic conditions at the study area intersections. The Holliston Department of Public Works indicated that a pavement overlay along Prentice Street in the vicinity of the site was completed in 2004.

#### No-Build Condition Traffic Volumes

Based on the average annual growth rate discussed above the existing 2005 traffic volumes were projected five years to 2010 No-Build Conditions. As the newly created soccer fields are seasonal and have varying schedules for use no additional traffic volumes were added to the existing measured traffic volumes to calculate the 2010 No-Build traffic volumes. In order to assess traffic conditions within the study area when the soccer fields are in full use a separate peak soccer conditions analysis was performed Section IV. Figure 3 depicts the projected 2010 No-Build Condition Peak Hour Volumes (without any additional soccer traffic volumes).

#### Assessment of Future No-Build Conditions

Capacity analyses were conducted for 2010 No-Build conditions for the weekday morning, weekday evening, and weekend mid-day peak hour traffic volume conditions for the study area intersections. The results of these analyses are shown in Table 5. A copy of these analyses worksheets is included in the Appendix.

From Table 5 it is evident that the study area intersections are expected to continue to operate at acceptable levels of service during the weekday morning, weekday evening, and weekend mid-day peak hour periods under future No-Build conditions. Some slight reductions in level of service are expected from existing conditions. This is a result of the added traffic volumes to account for the assumed background growth rate of the area, chosen as 3.5% as discussed previously.

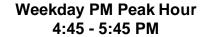
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I anie 5	/U1U NO-F	Ruild Conditions	<ul> <li>intersection i</li> </ul>	I EVELOT SELVICE	Summarv

Table 5. 2010 No-Build Conditions – Intersection Level of Service Summ							
	v .		Weekday Evening Peak		Saturday Mid-day Peak Hour		
			our				
Location/Movement	Delay (spv) <sup>a</sup>	$LOS^b$	Delay (spv)	LOS	Delay (spv)	LOS	
Marshall Street @ Prentice Street							
Prentice St. WB - LT	7.9	A	8.1	A	8.1	A	
Marshall St. NB - LR	12.3	В	13.5	В	12.6	В	

<sup>&</sup>lt;sup>a</sup> Average delay for specific intersection movement expressed in seconds per vehicle

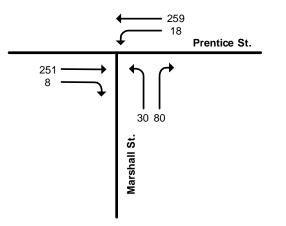
<sup>&</sup>lt;sup>b</sup> Level of Service

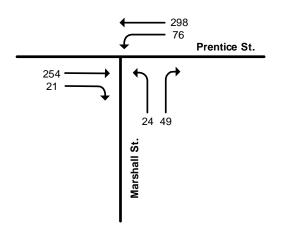


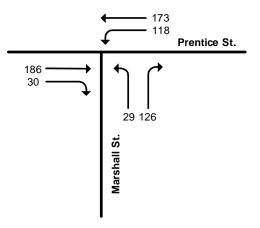


Saturday Peak Hour 11:00 AM - 12:00 PM



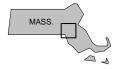






Not to Scale





2010 No-Build Conditions Traffic Volumes

Cedar Ridge Estates Holliston, Massachusetts

# Section IV Future Build Conditions

#### **Development Plan**

The proposed Cedar Ridge Estates project includes the development of 200 residential townhouse style condominiums. The condominiums will be constructed on currently undeveloped land on the southwest corner of the Prentice Street at Marshall Street intersection.

#### Site Access

Two site driveways on the west side of Marshall Street will provide access to the proposed development. Both site driveways will provide access to any point within the site. An emergency only access is also proposed at the southern end of the site. Within the site, the main site driveways are proposed at 22 feet wide with bituminous curbing. All of the proposed units will have individual driveways. There are no existing sidewalks on Marshall Street and sidewalks are not proposed within the site.

Proposed Site Driveway #1 @ Marshall Street

Proposed site driveway #1 intersects Marshall Street approximately 800 feet south of the intersection of Marshall Street at Prentice Street. Site driveway #1 initiates off the west side of Marshall Street, and provides access to any of the proposed units.

Proposed Site Driveway #2 @ Marshall Street

Proposed site driveway #2 intersects Marshall Street 1350 feet south of the intersection of Marshall Street at Prentice Street, and 550 feet south of the intersection of site driveway #1 at Marshall Street. This driveway is located on Marshall Street between existing residential properties. Site driveway #2 initiates off the west side of Marshall Street, and also provides access to any of the proposed units.

#### Sight Distance

A sight distance evaluation was performed for the proposed site driveway and for the intersection of Marshall Street at Prentice Street. Two separate sight distance criteria were considered in evaluating the location of the proposed site driveways and the existing Marshall Street @ Prentice Street intersection. Vehicles approaching the driveways to the site on Marshall Street and vehicles approaching Marshall Street at Prentice Street will require enough sight distance for stopping. Vehicles exiting from the site driveways or Marshall Street will require sufficient intersection sight distance to safely access the adjacent streets. Coler & Colantonio, Inc. conducted a sight distance inspection in the field and reviewed the proposed site plans to determine the available stopping and intersection sight distances. Discussions of each follow.

The adequacy of available sight distance is a function of the speed at which vehicles are approaching. In order to determine the adequacy of available sight distances, the vehicle speeds on the adjacent roadway was reviewed. The posted speed limit is 35 mph on Prentice

Street at the intersection with Marshall Street. There is no posted speed limit on Marshall Street. A 24-hour speed count performed on Prentice Street and on Marshall Street at the location of the site found that the average speed of vehicles was 36 mph on Prentice Street and 36 mph on Marshall Street. The 85<sup>th</sup> percentile speed (speed at which 85% of vehicle were at or below) was 40 mph on Prentice Street and 42 mph on Marshall Street. The 85<sup>th</sup> percentile speed is commonly used as a design speed for a roadway.

#### Stopping Sight Distance

The available stopping sight distance was measured in the field using an object 2.0 feet above the road surface at the primary site drive location and driver's eye height of 3.5 feet, according to AASHTO guidelines. An object height of 2.0 feet is representative of an object that involves risk to drivers that can also be recognized by a driver in time to stop before reaching it.<sup>1</sup>

Table 6 displays both the AASHTO recommended stopping sight distances and the available stopping sight distance as measured in the field approaching the site driveways. It can be seen from the table that the available sight distances at the intersection of Marshall Street at Prentice Street and the at the proposed site driveways exceed the minimum AASHTO recommended stopping sight distance for the posted speeds, average and 85<sup>th</sup> percentile speeds.

-

<sup>&</sup>lt;sup>1</sup> American Association of State Highway and Transportation Officials, *A Policy on Geometric Design of Highways and Streets*, AASHTO, 2001, Page 127.

**Table 6. Stopping Sight Distance** 

Location/Condition	Stopping Sight Distance (ft)
Marshall Street @ Prentice Street	
Recommended Sight Distance	
35 MPH (Posted Speed)	250 ft
36 MPH (Average Speed)	260 ft
40 MPH (85 <sup>th</sup> Percentile Speed)	305 ft

#### Intersection Sight Distance

Intersection sight distance is the distance required for vehicles exiting the site driveways to safely access or cross the adjacent roadway without interrupting the flow of traffic on the existing through street. The intersection sight distance at site driveways and at Marshall Street was measured from a point 14.5 feet outside the edge of the major-road travel way, which is within the range of AASHTO's<sup>3</sup> sight triangle vertex. At the intersection of Marshall Street the intersection sight distance looking left was also measured from a point 10 feet back from the travel way (approximate furthest point to which a vehicle can safely pull up).

Table 7 displays the AASHTO minimum intersection sight distance for both the posted speed limits and the average and 85<sup>th</sup> percentile travel speeds. The table also displays the existing intersection sight distance as measured in the field, exiting the site driveways onto Marshall Street, anticipating the driveways being constructed and site clearing and grading performed. Also included in the table is the intersection sight distance available exiting Marshall Street onto Prentice Street looking each direction without any improvements.

It is evident from the table that the measured intersection sight distances exceed the minimum AASHTO intersection sight distance for the posted speeds, average travel speeds, and 85<sup>th</sup> percentile speeds for the proposed Site Driveways. Two of the available intersection sight distances do not quite meet the AASHTO recommended distances for avoiding interruption in traffic flow of approaching vehicles on Marshall Street. Looking left out of Site Driveway #1 and looking right out of Site Driveway #2 the sight distances are limited by vertical curves in Marshall Street. Although these directions do not meet the recommended intersection distances they do meet the minimum distances for vehicles to stop and do not represent a safety concern.

Intersection sight distance was also evaluated at the existing Marshall Street at Prentice Street intersection. From Table 7, it is evident that the intersection sight distance looking right out of Marshall Street exceeds the minimum and recommended AASHTO intersection sight distance criteria for the posted speeds, average travel speeds, and 85<sup>th</sup> percentile speeds. At the intersection, both trees along Prentice Street and a vertical curve in Prentice Street limit the existing intersection sight distance for vehicles looking left out of Marshall Street. The minimum sight distance for posted speed and average speed looking left (west) is only satisfied from a driver's eye position 10 feet outside the edge of the major-road travel way instead of the recommended 14.5 feet. Even with pulling up to 10 feet outside the traveled way the available sight distance does not meet the 85<sup>th</sup> percentile speed minimum distance.

It is our observation that the sight distance at the intersection is an existing problem, and the proposed development will not worsen the sight distance condition.

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<sup>&</sup>lt;sup>3</sup> Ibid, Page 660.

**Table 7. Intersection Sight Distance** 

Table 1. Intersection digit	Intersection Sight
Location/Condition	Distance (ft)
Marshall Street @ Prentice Street	
Minimum Sight Distance <sup>a</sup>	
35 MPH (Average Speed)	250 ft
36 MPH (Average Speed)	260 ft
40 MPH (85 <sup>th</sup> Percentile Speed)	305 ft
Recommended Sight Distance	
35 MPH (Average Speed)	390 ft
36 MPH (Average Speed)	401 ft
40 MPH (85 <sup>th</sup> Percentile Speed)	445 ft
<b>Existing Sight Distance</b>	
Looking West (Left @ 14.5' back)	155 ft
Looking West (Left @ 10' back)	260 ft
Looking East (Right)	570 ft
Marshall Street	
Minimum Sight Distance <sup>a</sup>	
36 MPH (Average Speed)	260 ft
42 MPH (85 <sup>th</sup> Percentile Speed)	327 ft
Recommended Sight Distance	
36 MPH (Average Speed)	401 ft
42 MPH (85 <sup>th</sup> Percentile Speed)	467 ft
<b>Existing Sight Distance</b>	
Proposed Site Driveway #1	
Looking North (Left)	410 ft
Looking South (Right)	500 ft
Proposed Site Driveway #2	
Looking North (Left)	800 ft
Looking South (Right)	400 ft

<sup>a</sup>Source: AASHTO, Exhibit 9-55<sup>4</sup>

#### Sight Distance Summary

In summary the available stopping sight distance and intersection sight distance for the site driveway exceed the AASHTO recommended minimums for posted speeds, average speeds and 85<sup>th</sup> percentile speeds for most all approaches. At the intersection of Marshall Street at Prentice Street there is one case where intersection sight distance is below the minimum for the posted speed and average speed. Looking left out of Marshall Street, sight distance is limited by trees on the corner of Prentice Street and Marshall Street. The applicant owns the property on this corner and is willing to perform clearing and grading within the lines of sight of Marshall Street exiting traffic in order to improve the existing sight distance deficiency at the intersection.

<sup>&</sup>lt;sup>4</sup> Ibid, Page 665.

In order to assure safety in the future once the site is constructed, the proponent will maintain a line of site free of obstructions greater than 3.5 feet in height that would limit intersection sight distance at the proposed site driveways.

#### **Project Trip Generation**

The magnitude of site-generated traffic is directly related to the type of use and size of the proposed operation. Typically, the critical impact of a project on the area roadway network occurs during the peak hours of use of the surrounding roadway network. Because of the fact that the proposed development is residential the weekday morning (7:00 to 9:00 AM), weekday evening (4:00 to 6:00 PM) and weekend mid-day (11:00 AM to 1:00 PM) were selected as the peak hour analysis periods.

The proposed site used ITE methods to determine the number of vehicles that will be generated. For determining the trip generation ITE Trip Generation, 7<sup>th</sup> Edition was used. The Land Use Code selected was 230 Residential Condominium/Townhouse, which is representative of the proposed residential development.

#### Traffic Generation Methodology

Trip generation numbers for the proposed project are based on the methodology recommended in the ITE Trip Generation Handbook. The number of vehicles the residential development will generate is based on the number of dwelling units within the site. Using Land Use Code 230 selected above and the number of dwelling units the number of vehicles generated on a daily basis for each of the weekday morning and weekday evening peak hours can be determined.

The ITE Trip Generation Handbook cites different methods of determining trip generation based on the amount of data available for a particular land use. As recommended by the ITE Trip Generation Handbook the regression equation was used to determine the trip generation. This method was appropriate due to the fact that more than 20 data points were available and the number of apartments was within the range of the existing data. The regression equation is slightly more conservative, yielding higher estimates than average rates. The percentage of vehicles entering and exiting the site is also included in the ITE Trip Generation Land Use tables. Table 8 summarizes the unadjusted trip generation from the ITE methodology.

**Table 8. Trip Generation Summary** 

## $\frac{LUC~230,~Residential}{Condominium/Townhouse}~^{\underline{a}}$

Time Period/Direction	Trip Generation (vehicles)
Time I eriou/Direction	(venicles)
Average Weekday Daily	1158
Weekday Morning Peak Hour	90
Entering (17%)	15
Exiting (83%)	75
Weekday Evening Peak Hour	106
Entering (67%)	71
Exiting (33%)	35
Average Saturday Daily	1152
Saturday Mid-day Peak Hour	101
Entering (54%)	55
Exiting (46%)	46

<sup>&</sup>lt;sup>a</sup> Institute of Transportation Engineers, Trip Generation, 7<sup>th</sup> Edition, Washington, 2003.

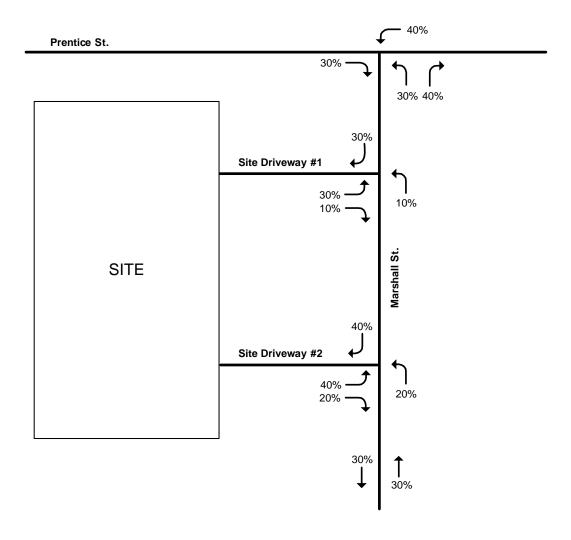
#### Trip Distribution and Assignment

Trip distribution pertains to the origin and destination of project related trips on the surrounding roadway network. The directional distribution of vehicles to and from the proposed residential project was based on a number of factors. These factors include existing travel patterns of adjacent roadways and logical travel patterns. The assignment of resulting percentages to/from local communities was based on existing travel patterns and logical travel routes and the assumption that the majority of drivers will seek the most efficient travel route to and from the site. The directional distributional percentages are presented in Figure 4.

Based on the distribution assumptions described, the total number of trips was assigned to the adjacent roadway network. Trip assigned volumes are presented for the weekday morning peak hour and weekday evening peak hour are presented in Figure 5.

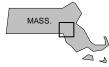
The peak hour project-related traffic included in the trip assignment diagrams was added to the 2010 No-Build condition peak hour traffic volumes. The resulting 2010 Build condition peak hour traffic volumes are presented in Figure 6.





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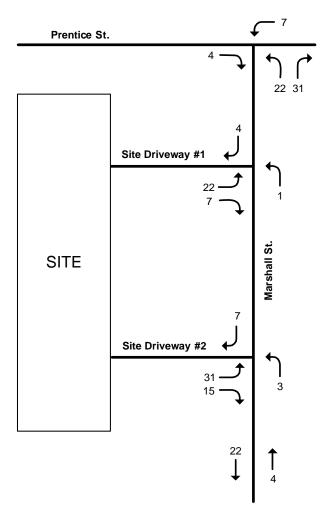




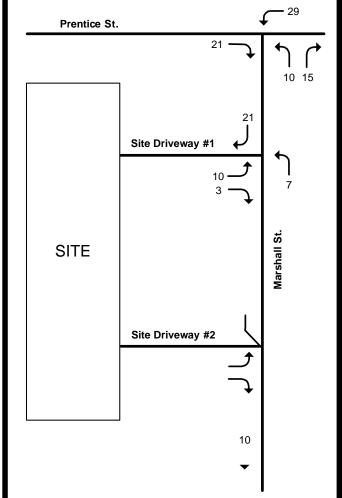
**Trip Distribution** 

Cedar Ridge Estates Holliston, Massachusetts

## Weekday AM Peak Hour 7:00 - 8:00 AM

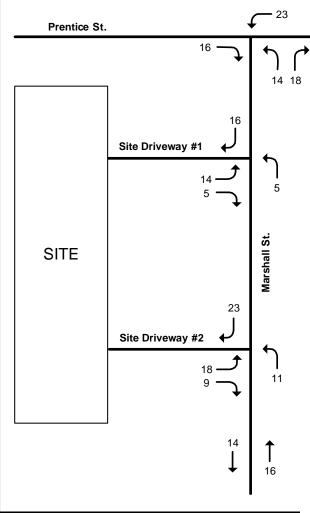


## Weekday PM Peak Hour 4:45 - 5:45 PM

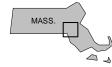


## Saturday Peak Hour 11:00 AM - 12:00 PM







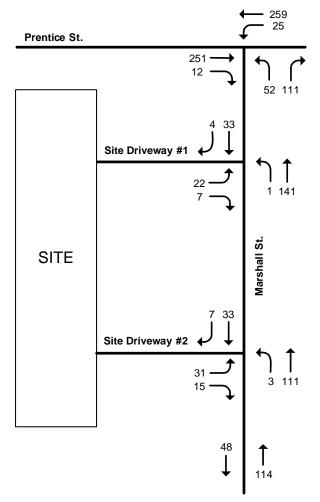


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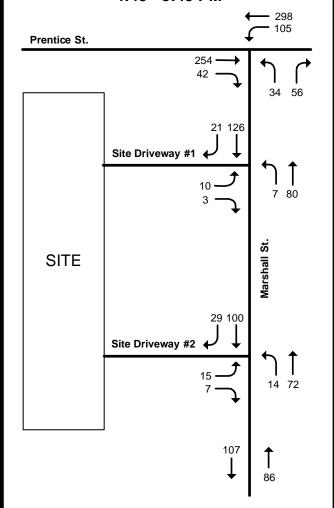
**Trip Assignment** 

Cedar Ridge Estates Holliston, Massachusetts

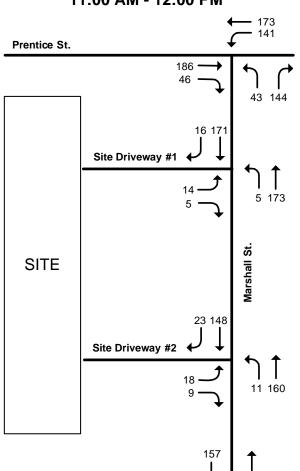
## Weekday AM Peak Hour 7:00 - 8:00 AM



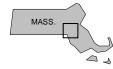
## Weekday PM Peak Hour 4:45 - 5:45 PM



## Saturday Peak Hour 11:00 AM - 12:00 PM







**Not to Scale** 

2010 Build Conditions Traffic Volumes

171

#### Assessment of Future Build Conditions

Capacity analyses were conducted for the weekday morning and weekday evening peak hour traffic volume conditions for the study area intersections. The results of the intersection capacity analyses are summarized in Table 9. The actual intersection and roadway capacity analyses are presented in the Appendix.

There was almost no change in level of service from No-Build Conditions to Build Conditions at the study area intersection. The only movement to have a change in LOS was the northbound movement on Marshall Street during the weekday evening peak hour. This approach only changed from a LOS "B" to LOS "C" due to a slight increase in delay at that approach. This change in LOS is due to the small number of vehicles added to the intersection and slightly increasing delay. In actuality, the additional delay will not be noticeable.

Table 9. 2010 Build Conditions – Intersection Level of Service Summary

Table 3. 2010 Build Conditions - Intersection Level of Cervice Cultima						
	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Mid-day Peak Hour	
Location/Movement	Delay (spv) <sup>a</sup>	$LOS^b$	Delay (spv)	LOS	Delay (spv)	LOS
Marshall Street @ Prentice Street						
Prentice St. WB - LT	7.9	A	8.3	A	8.2	A
Marshall St. NB - LR	13.9	В	15.5	C	14.6	В

<sup>&</sup>lt;sup>a</sup> Average delay for specific intersection movement expressed in seconds per vehicle

<sup>&</sup>lt;sup>b</sup> Level of Service

#### Assessment of Peak Soccer Field Use Conditions

An evaluation has been performed of the traffic impacts associated with the recently constructed (and the to be constructed) soccer fields near the southern end of the site on Marshall Street. The new soccer field development on Marshall Street consists of two new tournament size soccer fields and 4 practice fields with a parking area with 165 designated spaces and 65 overflow spaces. At the time of this study the two full size soccer fields have been completed are in use and the practice fields remain under construction.

The soccer fields will be a seasonal use with highest activity expected in the spring and fall. The peak times of the week expected for use are the late afternoons during the week and during the day on the weekends. The time when the impact to traffic from the soccer fields is expected to be greatest will be when back to back soccer games are played and all traffic from one game time will be leaving the complex and all traffic from the next game will be entering the complex. In order to assess the busiest anticipated traffic volume condition at the study area intersection of Marshall Street at Prentice Street a capacity analysis was performed assuming that the peak soccer period and the peak hour of non-soccer traffic occurred at the same time. Both the weekday PM and weekend mid-day peak hour periods were analyzed.

In order to determine the peak hour traffic volumes under peak soccer field use conditions the expected maximum soccer traffic volumes for two successive games was added to the 2010 Build Conditions Traffic Volumes. The maximum soccer field traffic volumes were assumed to be equal to the total number of parking spaces available (designated plus overflow spaces). It was assumed that a full 230 vehicles will enter and exit the soccer complex within the same peak hour period. These vehicle trips were then distributed to Marshall Street in the same manner as project related trips based on the existing travel patterns on Marshall Street. The resulting Peak Soccer Field Use Build Conditions Volumes are depicted in Figure 7.

Using the peak hour traffic volumes depicted in Figure 7 a capacity analysis was performed at the Marshall Street at Prentice Street intersection for the peak soccer field use condition. The results of the peak analysis are shown in Table 10. Capacity analysis worksheets are included in the Appendix.

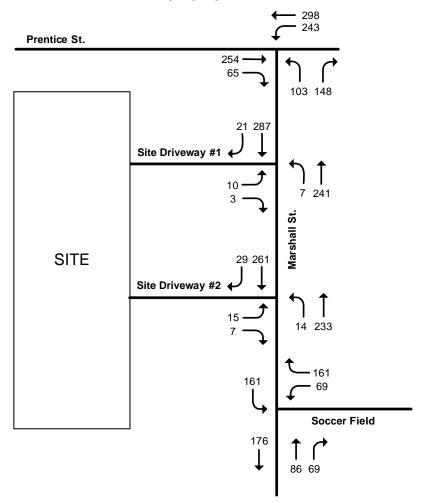
Table 10. 2010 Build Conditions – Intersection Level of Service Summary
Peak Soccer Field Use Conditions

i can doccer i icia doc dorialitorio			
Weekday Evening Peak Hour		Saturday Mid-day Peak Hour	
9.0	A	8.9	A
119.9	F	120.1	F
	Weekday Ev Hou Delay (spv)	Weekday Evening Peak Hour  Delay (spv) LOS  9.0 A	Weekday Evening Peak Hour  Belay (spv)  9.0  A  Saturday Mid Hou  Delay (spv)  September 1  A September 2  Belay (spv)  A September 3  Belay (spv)  September 3  Belay (spv)  September 4  Belay (spv)

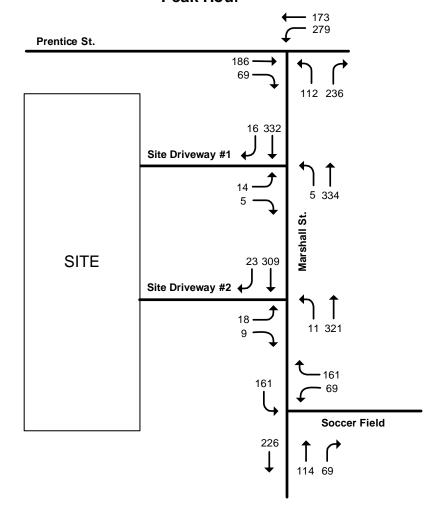
<sup>&</sup>lt;sup>a</sup> Average delay for specific intersection movement expressed in seconds per vehicle
<sup>b</sup> Level of Service



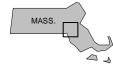
### Weekday PM Peak Hour 4:45 - 5:45 PM



## Weekend mid-day Peak Hour







Not to Scale

2010 Build Conditions with Soccer Field Traffic Volumes The results of the peak soccer conditions capacity analyses indicate that during a period when all of the parking spaces at the soccer complex are used for back-to-back soccer games there will be greater delay at the intersection of Marshall Street at Prentice Street. Prentice Street is expected to continue to operate at a LOS "A", however the Marshall Street approach is expected to operate at LOS "F" during this time. At these peak soccer traffic times the queue of vehicles at Marshall Street may reach as much as 15 vehicles during the worst 5% of the peak hour. A queue of this length is not expected to occur except during the busiest period of the peak soccer traffic times and would not extend to the proposed site driveways, which are over 700 feet from Prentice Street.

# Section V Conclusions/Recommendations

#### **Conclusions**

Coler & Colantonio has reviewed the traffic related impacts of the proposed Cedar Ridge Estates. Based on the findings of our study, it is our opinion that the development of the proposed residential condominiums at the site will have no effect on traffic or safety conditions on the roadways and intersections within the study area. The following is a bulleted summary of the findings regarding the proposed condominiums:

- The proposed Site Driveways providing access to the site along Marshall Street will have adequate sight distances, all exceeding AASHTO minimum requirements.
- At the intersection of Marshall Street at Prentice Street available Stopping Sight distance for approaching vehicles was found to exceed AASHTO minimum requirements. Trees and a vertical curve limit intersection sight distance for vehicles stopped on Marshall Street looking left. The applicant is willing to perform clearing and grading in order to improve the existing sight distance limitations at the intersection.
- Accident data indicated that the intersection of Marshall Street at Prentice Street had a below average crash rate and that the intersection is not prone to accidents.
- There are no significant changes in Level of Service to any study area intersections in any peak hour period due to the addition of project related trips. The only change is due to a borderline LOS being decreased by a less than two second change in average delay for the Marshall Street northbound approach to Prentice Street in the weekday PM peak hour.
- Site circulation was reviewed and it was determined that residents and emergency vehicles will be able to effectively maneuver throughout the site while accessing from one of two Site Driveways. A separate emergency vehicle access is also proposed.
- Although the level of service of Marshall Street is expected to be LOS "F" during the peak soccer traffic periods (time when back to back soccer games coincide with roadway peak hours) the queues expected on Marshall Street will not effect the proposed site driveways.

#### Recommendations

The following is a list of traffic and safety related recommendations relevant to the proposed residential condominium project:

• The added traffic associated with the development of the Cedar Ridge Estates is minor and does not necessitate performing any major traffic related improvements to the study area roadways or intersections due to capacity or safety reasons.

- Within the site, stop signs, stop lines and crosswalks should be incorporated at all intersections of main driveways.
- The applicant should commit to providing and maintaining clear lines of sight at each of the proposed Site Driveways on Marshall Street. No obstructions to sight distance (typically objects greater than 3.5 feet in height) should be placed within the site lines of approaching or exiting vehicles. Also, adequate clearing of existing vegetation where applicable should be performed to ensure clear lines of sight.
- In order to improve sight distance at the intersection of Marshall Street at Prentice Street, trees, low growth and any other obstructions within the line of sight on the southwest corner of the intersection should be cleared.
- Advance warning signs of an intersection ahead with advisory speed plates should be installed on Prentice Street approaching Marshall Street.
- Speed limit signs should be installed on Marshall Street, as there are currently no posted speed limits.
- Advance warning signs with advisory speed plates should be added to locations on Marshall Street where horizontal curves limit available sight distance.

## **Appendix**

- Traffic Counts
- Capacity Analyses
- Crash Rate Worksheets
- MHD Seasonal and Growth Tables
- ITE Trip Generation
- Marshall Street Inventory

Cedar Ridge EstatesTraffic StudyHolliston, MAOctober, 2005

## **Traffic Counts**

Cedar Ridge EstatesTraffic StudyHolliston, MAOctober, 2005

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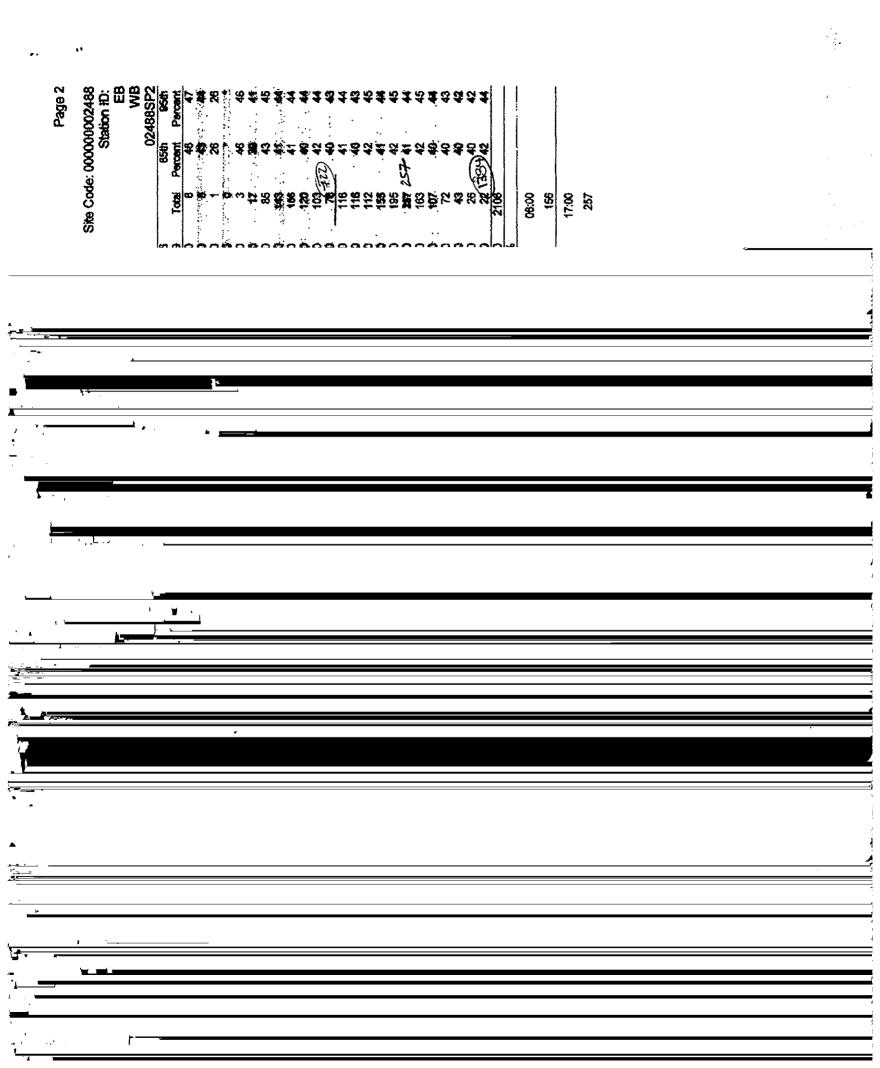
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Grand Total	0	343	25	104		37	46	050		
Approh %	0.0				0		16	359	0	884
Appel 70		83.2	6.8	73.8	0.0	26.2	4.3	95.7	0.0	
Total %	0.0	38.8	2,8	11.8	0.0	4.2	1.8	40.6	0.0	

		Avh 8 From	East			Maraha From	i Street South			Ash Si From V			
Start Time Peak Hour From 07:0	Right	Thru	Lot	App. Total	Footie	Thou	Left	App. Total	Foliant	Thru	LaR	App. Tobal	Int. Total
foto	2 AM to U5:45	AM - Peak	1 of 1							<del></del>			
intersection	07:00 AM												ł
Volume	0	185	13	198	57	n	21	78	a	179	•	185	1 404
Percent	0.0	93.4	6.6		73.1	0.0		70			- 0	100	461
07:30 Volume	ñ						26.9		3.2	96.8	0.0		•
	ν	55	2	57	13	0	7	20	3	43	0	46	123
Pesk Factor								I					0.937
High Int.	07:30 AM				07:00 AM			į	07:45 AM				0,557
Volume	O	55	3	57	21	^	•	,	V1> ////		_		ļ
Peak Factor	•	~~	4		21	U	3	24	1	56	O	57	1
1 WISK   EVEN				0.868	ľ			0.813				0.811	1

TDC
Transportation Data Corporation
RO, BOX 734 Nucles, MA 01780
Office: 608-681-1610 Fex: 808-681-1229

S: Marshall Street E/W: Ash Street

City, State: Holliston, MA Client: C&C/J. Morgan File Name: 02488AA Site Code: 00000000 Start Date: 05/01/2003

Page No :1

				Groups Prin	ied- Trucks			1 age	140 : 1	
Start Time		ush Street rom East		Ma	rahell Street rom South			Ash Street		
Ocal Isla	Right	Titru	Left	ftight	Thru		***************************************	rom West		
07:00 AM	0	1	0	~		Left	Rigist	Thru	Left	Int. Total
07:15 AM	Ó	4	71	Y	Ü	0	G	0	0	1
07:30 AM	ň	;		3	Q	0	0	2	a	,
07:45 AM	ŏ	<del>4</del>	9	Ō	0	0	0	2	õl	Ĭ
Total	<u>v</u>	<u> </u>		0		0	Ğ	Ä	ň	7
1000	U	4	· 1	1	0	0	n	8	<del> </del>	
00.00 434	_					-,	. •	0	0	14
08:00 AM .	D.	3	0 (	1	^	A 1		_	_	
08:15 AM	0	2	اة	΄.	ŭ	Υ	Q.	2	0	В
08:30 AM	Ň	7	21	Ų	U	0	1	0	o l	Ā
08:45 AM	Ž	· ·	7	0	0	וס	٥	4	1 6	•
	U	1	01	2	0	Ŏ.	¥ .	à	Ž I	3
Total	0	8	1	2	<del></del>			<u> </u>	U	7
	· ·	_	• ;		O	0	2	6	0	20
Grand Yotal	ń	12	21		_					· ·
Approh %	~~		- 4	4	0	0	2	14	0	34
7444CE 20	0.0	85.7	14.3	100.0	0.0	0.0	12.5	87.5	- 1	<b>9</b>
Total %	0.0	35,3	5.8	11.8	0.0	0.0			0.0	
			1		0.0	0.0	5.9	41.2	0.0	

Start Time	Right	From	East			Marshall From S				Ash St From V			
mak Hour From 07:00	AN IS OF 45	AM - Deal	Left	App. Total	Fögfrit	Thru	Ler	App. Total	Right	Thru	Let	App. Total	int. Total
Intersection	08:00 AM	*****	× 1 01 1		1 .				······································				HIL TOUR
Volume	0	8	1	9	4	۸	^	ام	_	_		1	
Percent	0.0	88.9	.11.1	•	100.0	0.0	0.0	3	2 2	- 6	0	8	20
08:45 Volume	-0	1	o	1	2	0.0		_	25.0	75.0	0.0		
Peak Factor			•	•	•	v	0	2,	[ <b>T</b>	3	0	4	7
High Int. Volume	08:00 AM	_	_		08:45 AM				08:45 AM			ſ	0.714
Peak Factor	v	3	0	3	2	0	0	2	1	3	Ð	4	
				0.750				0.375		•	•	0.500	

Transportation Data Corporation P.O. Box 734 Nation, MA 01780 Office: 506-551-1610 Fax: 508-651-1229

S: Marshall Street E/W: Ash Street

City, State: Holliston. MA

File Name: 02488AAA

Site Code : 00000000 Start Date : 05/03/2003

Client: C&C/J. Morgan

Pag

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			(	kroups Printed-						
		ah Strant. rom East			shell Street om South			sh Street rom West		
Start Time	Föght	Teru	Left	Right	Thru	Let	Right	Thru	Lant	INC. TOWN
11:00 AM	0	37	28	28	0	3	7	38	0	139
11:15 AM	O.	26	26	25	0	8	4	28	0	115
11:30 AM	0	30	19	21	0	5	6	36	0	117
11:45 <b>AM</b>	0	31	13	16	9	6	4	31	O	101
Total	Ö	124	84	90	0	20	21	133	0	472
12:00 PM	0	53	18	8	0	5	5	27	0	116
12:15 PM	C	44	15	19	Q	5	2	28	0	113
12:30 PM	O.	50	30	21	0	8	2	22	0	133
12:45 PM	0	30	20	20	0	6	5	26	0	107
Total	O	177	63	68	O	24	14	103	0	469
Grand Total	0	301	187	158	0	44	35	235	0	941
Approh %	0.0	64.3	35.7	78.2	0.0	21.8	12.9	87.1	0.0	
Total %	0.0	32.0	17.7	16.8	0.0	4.7	3.7	25.1	0.0	

		Ash 8 From				Marshal From				Ash St From V	Vest		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Len	App. Yolal	Right	Thru	Leaft	App. Total	PR. 100
wik Hour From 11:00	AM to 1246	PM - Page	C1 0f 1	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	A								
Intersection	11:00 AM												
Volume	0	124	84	208	90	0	20	110	21	133	Q	154	472
Percent	0.0	59.6	40.4		81.8	0.0	18.2		13.6	86.4	0.0		ľ
11:00 Volume	0	37	26	63	28	0	3	31	7	38	0	45	139
Pack Factor					1				•			1	0.849
High int.	11:00 AM				11:00 AM				11:00 AM				
Volume	¢	37	26	63	28	0	3	31	7	38	0	45	1
Peak Factor				0.825			•	0.887				0.856	<b>!</b>

	<b>TD</b> C Transpor	tation Data Corporation RO, BOX 734 Nation, MA 01760	File Merce · 02488A A A	
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Transportation Data Corporation P.O. Box 734 Phillips, MA 01780 Office: 508-651-1810 Fax: 508-651-1228

ATR : Marshall Street south of

Location

Site: 02488 Date: 05/01/03

Client : C&C/J. Morgan Direction: NB

: Prentice Street, Holliston, MA

**************************************		4 4 4	****				<u> </u>								
Begin	Total	1-14	15-19	20-24	25-29	30-34	35-39	40-44	45-49	50-54	55-59	60-64	65-69	70-99	AVE
Time		MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	
12:AM	1	0	0	0	0	0	0	J	0	0	O	0	0	0	42
01:00	1	0	0	0	0	0	1	0	0	0	0	0	0	0	37
02:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00	0	0	• 0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	1	0	0	0	G	0	0	I	Q.	0	0	0	0	0	42
05:00	9	0	0	0	1	1	3	3	1	0	0	0	0	Û	38
06:00	45	0	0	0	3	7	19	14	2	0	0	0	. 0	0	38
07:00	72	0	1	3	4	9	22	28	5	0	0	0	0	0	38
08:00	71	0	0	2	3	17	28	18	3	0	0	0	0	0	37
09:00	30	0	0	0	0	3	15	9	3	0	0	0	0	0	39
10:00	17	0	0	0	2	3	5	7	0	0	0	0	0	0	37
11:00	. 15	0	0	0	1	2	6	5	i	0	6	0	0	0	38
12:PM	20-	0	0	1	0	4	10	3	2	0	0	0	0	0	37
01:00	21	0	0	0	2	4	8	6	į	0	0	0	0	0	37
02:00	36	0	0	2	6	5	13	8	2	0	0	0	0	0	35
03:00	34	0	0	0	3	6	14	10	1	0	Ó	0	0	Ò	37
04:00	38	0	0	2	į	7	16	9	3	0	0	0	0	0	37
05:00	43	0	I	0	0	5	21	13	2	1	0	0	0	0	38
06:00	35	0	0	0	0	8	14	7	5	1	0	0	0	0	39
07:00	30	0	0	1	1	4	16	4	4	Ö	0	0	0	0	38
08:00	15	0	0	0	2	6	6	1	0	0	0	0	0	0	34
09:00	17	0	0	0	1	4	6	5	1	0	0	0	0	0	37
10:00	10	0	0	1	2	1	4	2	0	0	G	0	٥	0 -	34
11:00	2	.0	0	0	0	0	2	0	0	0	0	0	0	0	37
Daily	563	0	2	12	32	96	229	154	36	2	0	0	0	0	37
Totals				*											
Percent		0.0	0.4	2.1	5.7	17.1	40.7	27.4	6.4	0.4	0.0	0.0	0.0	0.0	
of Total						0117	****	<b></b>		***	***		•••	•••	
Percentile S	peods	10%	1:	5%	50%	85%	90%								***
	-	30.6		2.0	38.1	43.5	44.4								

% in pace 68.9

Speed Exceeded 45 MPH 55 MPH 65 MPH Percentage 6.7 0.0 0.0 Totals 38 0 0 Transportation Data Corporation PO. Box 734 Nucles, MA 01760 Office: 508-651-1510 Fex: 508-651-1229

ATR : Marshall Street south of

Location : Prentice Street, Holliston, MA

02488 Site: Date: 05/02/03

Client : C&C/J. Morgan Direction: NB

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Begin	Total	1-14	15-19	20-24	25-29	30-34	35-39	40-44	45-49	50-54	55-59	60-64	65-69	70-99	Ave
Time		MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MOPH	MPH	MPH	MPH	
12:AM	2	0	0	0	1	——i"	······································	0	0	0	0	0	0	Ö	30
01:00	1	0	0	0	0	0	0	1	0	Ô	0	ō	0	Ò	42
02:00	1	0	0	0	0	i	0	Ö	0	0	0	0	0	Ö	32
03:00	Ð	0	0	0	0	0	0	0	0	0	0	0	0	Ó	0
04:00	1	0	0	0	0	0	Ó	1	0	Ō	0	0	0	Ö	42
05:00	6	0	0	0	0	į	2	2	1	o	Ō	Û	ō	Ö	40
06:00	48	0	0	0	2	6	22	13	3	2	Ó	0	0	0	39
07:00	73	0	0	0	0	14	31	18	7	2	1	0	0	Ó	39
08:00	60	0	1	0	0	3	28	26	1	1	0	0	0	0	39
09:00	31	0	1	1	0	5	11	9	3	0	Ö	1	0	Ó	38
10:00	32	0	0	0	1	5	10	13	3	0	0	Ö	Ó	Ó	39
11:00	29	0	1	1	1	5	13	7	1	0	0	0	0	0	36
12:PM	34	0	0	1	1	14	12	6	Ö	Ò	Ö	Ò	0	Ó	35
01:00	28	0	0	0	1	7	13	4	3	Ó	Ó	Ô	0	Û	37
02:00	25	Û	1	Ô	ì	4	10	6	3	ŏ	Õ	ō	ŏ	ŏ	37
03:00	27	Ō	ō	Ò	Ö	6	12	ž	2	ŏ	ŏ	ō	ŏ	ō	38
04:00	49	Ö	ė	0	4	15	17	7	4	ì	Ŏ	ĭ	Ŏ	ŏ	37
05:00	47	0	0	Ó	2	16	21	8	ò	ō	ő	ò	ŏ	Ö	36
06:00	33	0	1	0	0	6	17	7	ž	ō	ŏ	Ď	Ö	á	37
07:00	31	C	0	0	3	10	11	7	0	Ö	ò	Ó	Ö	Ò	36
08:00	24	O.	0	0	4	6	11	3	ō	Ò	Ö	Ö	ō	Õ	35
09:00	18	0	0	1	0	6	9	2	Ö	Ö	Ŏ	ò	Ō	Ó	35
10:00	11	0	0	1	0	0	7	2	1	0	0	Ö	Ö	0	37
11:00	7	0	0	0	0	2	3	2	0	0	Ó	Ō	0	Ó	37
Daily	618	0	5	5	21	133	260	151	34	6	1	2	0	0	37
Totals															
Percent		0.0	0.8	0.8	3.4	21.5	42.1	24.4	5.5	1.0	0.2	0.3	0.0	0.0	
of Total															
Percentile S	ipeeds	10%	1:	5%	50%	85%	90%								
		414	•	• •		48 4									

31.2 32.3 37.8 43.4 44,4

10 MPH Pace Speed: 35 - 45 Number in pace 411 % in pace 66.5

7.0 Percentage ŧ 0.5 0.0 Totals 43 3 0



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Transportation Data Corporation Po. Box 734 No. 80 No. 1740 Office: 500-951-1610 Fex: 500-951-1220

ATR

: Marshall Street south of

TDC

Site: Date:

02488 05/01/03

Location

: Prentice Street, Holliston, MA

D	irc	cti	on:	SB	

Client	: C	&C/J. M	ORESTO			D	irection:	SB						<del></del>	
logis	Total	1-14	15-19	20-24	25-29	30-34	35-39	40-44	45-49	50-54	55-59	60-64	65-69	70- <del>99</del>	Avg
ime		MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	
12:AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	27
01:00	ì	0	0	0	1	0	0	0	0	0	0	0	Û	v	27 32
02:00	1	0	0	0	0	1	0	0	0	0	0	0	Ü	v	
03:00	0	0	0	0	0	0	0	0	0	0	0	0	9	,	0
04:00	0	0	0	0	0	0	0	0	0	0	0	Û	U	v	0
05:00	3	0	0	1	2	0	0	0	0	0	0	Q.	v	0	25 37
06:00	7	0	0	0	0	2	3	2	0	0	0	Ü	0	0	35
07:00	23	3	0	0	1	8	8	3	Ţ	1	¢	Ü	0	0	
08:00	22	0	2	2	0	7	11	0	0	0	0	0	0	0	32
09:00	18	ó	0	0	0	6	12	0	0	0	Q	U	U	0	35
10:00	20	ō	Ō	1	3	7	8	1	Q	0	0	0	0	0	33 36
11:00	13	Ŏ	Ò	1	0	2	8	2	0	0	0	Ü	Ū	0	33
12:PM	19	1	0	1	2	5	7	2	1	0	Q	Ü	0	. 0	35
01:00	25	Ö	0	1	3	8	9	3	0	1	0	0	V	0	32
02:00	34	Ŏ	i	2	5	16	8	2	0	0	0	0	0	9	34
03:00	42	ò	Ō	1	5	14	18	4	0	0	0	0	U	υ 0	37
04:00	52	ò	ò	0	4	13	22	10	3	0	0	Ü	Ü	Ŏ	35
05:00	58	ō	1	1	3	17	27	8	1	0	0	Ų	v	0	35
06:00	70	ñ	Ö	Ð	5	26	29	8	0	2	0	U	U	V	33
07:00	37	ŏ	0	0	6	18	10	3	0	0	0	Ű	o o	0	34
08:00	30	ō	Ö	0	5	10	11	3	1	0	0	Ų.	Ų	Ň	36
09:00	33	ō	0	0	1	12	14	5	1	0	0	V	0	0	40
10:00	8	Ó	0	0	0	2	2	2	1	1	0	0	0	ŏ	37
11:00	7	0	0	0	0	2	3	2	0	0	0	0	0	0	35
Deily	524	2	4	11	47	176	210	60	9	5	0	0	U	v	75
Totals															
Percent of Total		6.4	0.8	2.1	9.0	33.6	40.1	11.5	1.7	1.0	0.0	0.0	0.0	0.0	

10% 15% 50% 85% 90% Percentile Speeds 39.9 41.8 30.4 35,5 28.8

10 MPH Pace Speed : 30 - 40 386 Number in pace

% in pace 73.7

55 MPH 65 MPH Speed Expeeded 45 MPH 0.0 0.0 2.7 Percentage 14 Totals

Transportation Data Corporation
PO. Box 734 Number, MA 01790
Office: 808-861-1610 Fex: 508-961-1229

ATR : Marshall Street south of

: Prentice Street, Holliston, MA

Location

Site: 02488 05/02/03 Date:

		-	HAAN TIN	TITOMOGT! TA								سق	au.	٧.	10203
Client	: C	&C/J. M	orpan			I	Direction:	SB							
Ведія	Total	1-14	15-19	20-24	25-29	30-34	35-39	40-44	45-49	50-54	55-59	60-64	65-69	70-99	Ave
Time		MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	
12:AM	3	0	0	0	1	1	1	0	0	0	0	0	0	0	32
01:00	4	0	0	0	0	2	1	1	0	0	0	0	0	0	36
02:00	2	0	0	0	0	1	0	1	0	0	0	0	Ō	0	37
03:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	0	0	0	0	Ġ	Ô	0	0	0	Ō	Ó	0	Ô	0	0
05:00	I	G	0	1	0	٥	0	0	0	0	0	0	0	0	22
96:00	5	0	0	0	0	2	2	3	0	0	Ó	0	0	0	36
07:00	29	Į	0	1	4	8	11	4	0	0	0	0	0	0	33
08:00	16	0	1	1	1	7	6	0	0	0	0	0	0	0	32
09:00	20	1	0	1	3	8	5	2	0	0	0	0	٥	0	32
10:00	17	Đ	0	0	1	8	6	2	0	0	0	0	0	0	35
11:00	22	0	0	2	2	7	8	2	1	0	0	0	0	0	34
12:PM	36	0	Ó	Ö	5	17	6	8	ô	0	0	0	0	0	34
01:00	32	0	I	Ó	2	15	11	3	0	0	0	0	0	0	34
02:00	40	0	0	i	2	12	12	10	2	1	0	0	0	0	37
03:00	56	0	0	Ö	13	19	14	7	3	0	0	Ó	0	0	34
04:00	67	0	0	0	10	22	18	11	2	1	0	1	2	0	36
05:00	81	0	0	0	6	36	30	8	1	0	0	0	0	0	35
06:00	39	0	0	1	2	13	16	6	1	0	0	0	0	0	35
07:00	37	0	0	3	1	11	16	6	2	0	0	0	0	0	36
08:00	23	0	1	1	2	6	7	5	0	0	1	0	0	0	35
09:00	25	0	0	1	3	10	7	4	0	0	0	0	0	0	34
10:00	7	0	0	0	0	1	3	3	0	0	0	0	0	0	38
11:00	9	0	0	0	1	2	4	J	0	1	. 0	0	0	0	37
Daily	571	2	3	11	59	208	184	85	12	3		1	2	0	35
Totals															
Percent		0.4	0.5	1.9	10.3	36.4	32.2	14.9	2.1	0.5	0.2	0.2	0.4	0.0	
of Total															

10% 15% 50% 85% 90% Percentile Speeds 28.6 30.3 35.1 41.1 42.8

10 MPH Pace Speed : 30 - 40 Number in pace 392 % in pace 68.7

Spood Exceeded **45 MPH** 55 MPH 65 MPH Percentage 3.3 0.7 0.4 Totals 19 2 Transportation Data Corporation P.O. Box 734 Nucle, MA 01760 Office: 608-651-1610 Fex: 608-651-1229

ATR

: Marshall Street south of

Site: Date:

02488 05/03/03

Location

: Prentice Street, Holliston, MA

&C/J. Morgan	Direction: S
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85% 90% 10% 15% 50% Percentile Speeds 41.3 34.4 39.7 28.0 30.0

30 - 40 10 MPH Pace Speed: 619 Number in page

72.3 % in pace

55 MPH 65 MPH 45 MPH Speed Exceeded 0.2 0.1 2.0 Percentage 17 2 ì Totals

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Page 1 03442Bvolume Site Code: 03442

Marshall Street

south of Prentice Street City, State: Holliston, MA

	<u>&amp;C/J. Mo</u> i			11	Tatala		IB	Hour	Totals	Combin	ed Totals
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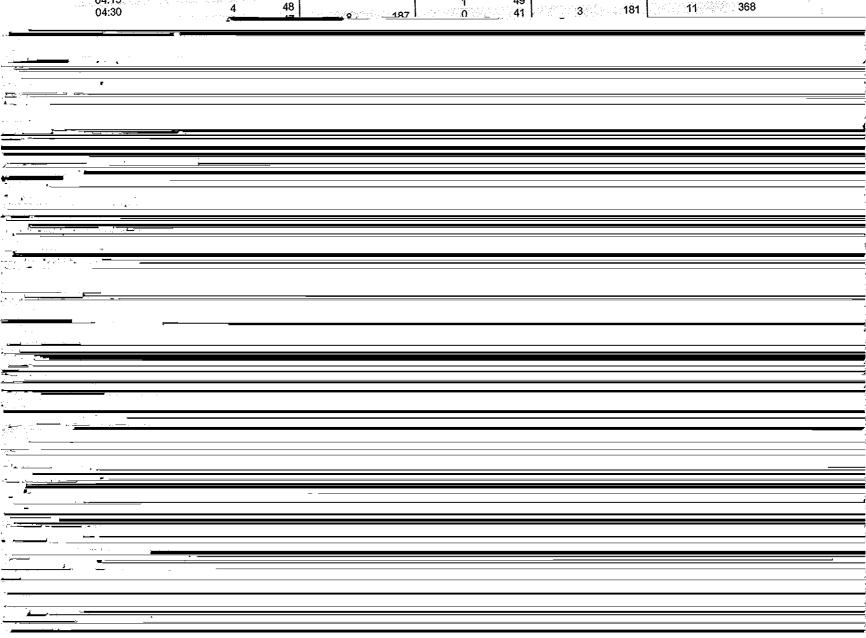
Transportation Data Corporation P.O. Box 334 Wakefield, MA 01880 Tel. (781) 587-0086 Fax (781) 587-0189 Cell (781) 316-4663 E: mperone1@comcast.net

Page 1 03442Avolume Site Code: 03442

Prentice Street west of Marshall Street

City, State: Holliston, MA

Client: Ca	<u>&amp;C/J. Mor</u>	gan		Hour	Totals	V	VB	Hour	Totals	_	ed Totals
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Page 1 03464Avolume Site Code: 03464

Marshall Street south of Prentice Street City, State: Holliston, MA

Client: C&	&C/I. Mo	rean											
Start	<u>~~// J+ *+*\</u>	\$B				NB				ombined	D.M	1	14-Oct-05 Fri
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Total													
Peak	07:30		04:00		07:00		05:15		07:00		04:00		
Vol.	44		80		101		54		133		129		
P.H.F.	0.846		0.800		0.789		0.844		0.792		0.787		

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Page 2 03464Avolume Site Code: 03464

Marshall Street south of Prentice Street City, State: Holliston, MA

lient: C&	<u>cC/J. Mo</u>	rgan								b. b. c - l		46.7	oct-05
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01:45	0	0	10	43	0	2	10	42	0	2	20	<b>6</b> 5	
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P.H.F.	0.917		0.922		0.044		0.700		0.07				

### Transportation Data Corporation

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Marshall Street south of Prentice Street City, State: Holliston, MA

Client: C&		rgan										16-Oc	
Start	•	ŚB			4.14	NB	P.M.		A.M.	Combined	P.M.	16-00 Sur	
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08:45	17	35	6	19	13	32	2	12	30	67	8	31	
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			0		22		1		32		1		
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11:00	12		4						45		1		
11:15	12		0		33		1						
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11:45	15	55	1	7	15	76	0	4	30	131	1	11	
Total	172		550		323		510		495		1060		
Percent	34.7%		51.9%		65.3%		48.1%						
Day Total		72	2			83	33			15	55		
10001													
Peak	11:00		02:45		10:30		03:15		11:00		03:15		
Vol.	55		112		89		122		131		209		
P.H.F.	0.809		0.824		0.674		0.545		0.728		0.679		

Transportation Data Corporation P.O. Box 334 Wakefield, MA 01880 Tel. (781) 587-0086 Fax (781) 587-0189 Cell (781) 316-4663 E: mperone1@comcast.net

Page 4 03464Aclass Site Code: 03464

Marshall Street south of Prentice Street

NB													> 0 A. J	Niet	
Start		Cars &	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 AxI	Not	T-1-1
Time	Bikes		Long	Buses	6 Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classe	Total
10/14/0						_	_			•	0	0	٥	0	3
5	0	2	1	0	0	0	0	0	Ŏ	ý	Ŏ	×	ŏ	ň	ň
01:00	0	0	0	0	0	0	Ō	Ŏ	ŭ	Ņ	Ŏ	ŏ	ň	ň	ň
02:00	0	0	o o	0	0	0	Ŏ	Ü	ŭ	Ŏ	ŭ	ň	ň	ŏ	1
03:00	0	1	Ō	0	0	O O	Û	0	ŏ	Ŏ	ŏ	ň	ň	ň	ó
04:00	0	0	0	0	0	U	U	0	0	0		Š	ŏ	ň	13
05:00	0	8	4	0	0	1	0	0	0	U	Ū	0	0	0	
06:00	0	33	13	0	0	0	0	1	0	0	O	Ü	U	Ų	47
07:00	Á	68	26	1	6	0	0	0	0	0	0	o o	0	0	101
08:00	ň	37	20	1	1	1	0	0	0	0	0	0	0	0	60
09:00	ň	22	16	Ó	1	0	0	0	0	0	Ō	ō	0	0	39
10:00	ŏ	13	4	Ö	0	1	0	1	0	0	0	0	0	Ŏ	19 22
11:00	ŏ	7	13	0	2	0	0	0	0	0	0	0	0	Ū	
12 PM	ň	16	9	1	2	2	0	0	0	0	0	0	0	0	30
13:00	ň	16	11	1	4	2	0	0	0	0	0	0	0	0	34
	4		9	ò	Á	ñ	Ò	0	0	0	0	0	0	0	31
14:00	1	17 16	16	ŏ	7	ň	ŏ	ŏ	ŏ	ŏ	Ó	0	0	0	32
15:00	U		17	2	•	Ď	ň	ñ	ń	0	0	0	0	0	49
16:00	0	29		1	- 4	0	^	0	0	Ŏ	ò	0	0	0	52
17:00	Q.	35	16	0	1	0	V	ŏ	ŏ	ň	ŏ	ŏ	ŏ	ō	41
18:00	0	31	7	Q	3	, v	V	Ŏ	ŏ	ŏ	ŏ	ŏ	ŏ	ŏ	33
19:00	0	23	9	0	1	Ŏ	V	V	Ň	ň	ŏ	ŏ	ŏ	õ	14
20:00	0	9	5	Ŏ	Ŏ		, ,	ŏ	ň	ň	ŏ	ŏ	Ŏ	Ó	8
21:00	0	5	3	0	4	. v	0	0	ň	ŏ	ŏ	ŏ	Ŏ	ō	19
22:00	0	12	6	V	1	0	0	ň	ň	ŏ	ŏ	ŏ	Ō	Ö	10
23:00	<u>o</u>	8	202	5	28	7	Ö	2	Ö	0	Ŏ	0	0	0	658
Total	<u>1</u>	408	207	^ ^~	4 20	4 40/	0.00%	บระ	∿00€	n n%	0.0%	0.0%	0.0%	0.0%	

AM		07:00	07:00	07:00	07:00	05:00	06:00	07:00
Peak Vol.		68	26	1	6	1	1	101_
РМ	14:00	17:00	16:00	12:00	13:00	12:00		17:00
Peak Vol.	11	35	17_	1_	4	22		52

## Transportation Data Corporation

P.O. Box 334 Wakefield, MA 01880 Tel. (781) 587-0086 Fax (781) 587-0189 Cell (781) 316-4663 E: mperone1@comcast.net Page 5 03464Aclass Site Code: 03464

Marshall Street

south of Prentice Street City, State: Holliston, MA

NB	-														
Start		Cars &	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Ax1	6 Axle	>6 Axl	Not	
Time	Bikes	Trailers	Long	Buses	6 Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classe	Total
10/15/0								•			_	_			_
5	0	4	1	0	0	0	0	0	0	0	0	0	0	Ō	5 2
01:00	0	2	0	0	0	0	0	0	0	0	0	0	0	0	3
02:00	Ó	2	1	0	0	0	0	0	0	0	0	0	0	0	3
03:00	0	1	1	0	Ō	0	0	0	Ŏ	0	Ů.	V	0	0	1
04:00	0	1	0	0	0	0	0	0	0	0	0	0	ő	ŏ	3
05:00	0	2	1	0	0	0	0	0	0	•	0	•	ő	ŏ	11
06:00	0	6	4	0	o	0	0	7	0	0	0	0	Ö	ŏ	24
07:00	0	16	6	0	2	0	0	0	0	0	0	ò	ŏ	ŏ	25
08:00	0	15	9	0	1	0	0	0	0	-	•	-	0	ŏ	39
09:00	1	27	8	0	3	0	0	0	0	0	0	0		-	54
10:00	1	41	12	0	0	0	0	0	0	0	0	0	0	0	
11:00	0	31	12	0	2	1	0	1	0	0	0	0	0	0	47
12 PM	0	29	10	0	2	0	0	0	0	0	0	0	0	0	41
13:00	1	29	12	0	0	0	0	0	0	0	0	0	0	0	42
14:00		24	13	ŏ	ĭ	Ŏ	ò	Ō	0	0	0	0	0	0	39
	0	18	7	ō	1	1	0	0	0	0	0	0	0	0	27
15:00	0	26	8	ŏ	3	Ó	ō	0	0	0	0	0	0	0	37
16:00	•			0	4	0	ŏ	ŏ	ō	0	0	0	0	0	34
17:00	0	18	15 · 7	0	Ö	ŏ	ő	ő	ŏ	ŏ	ō	Ö	0	0	24
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19:00 20:00	V	18	5	Ô	ň	ŏ	ŏ	ō	Ò	Ó	0	0	0	0	23
20:00	0	9	2	ŏ	ŏ	ŏ	ŏ	Ŏ	0	o	0	0	0	0	11
22:00	0	21	5	ŏ	ĭ	ŏ	Ö	0	0	0	0	0	0	O.	27
23:00	ŏ	11	3	ŏ	Ó	ō	0	0	0	0	0_	0_	0	<u>o</u>	14
Total	4	382	145	0	17	2	0	2	0	0	0	0	0	0	552
Percent	0.7%	69.2%	26.3%	0.0%	3.1%	0.4%	0.0%	0.4%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
, croom	0 / 0														
AM	^^ ^^	40.00	10:00		09:00	11:00		06:00							10:00
Peak	09:00	10:00				11.00									54
Vol.	1	41	12		3	1_		1							
PM	13:00	12:00	17:00		16:00	15:00									13:00
Peak					3	4									42
Vol.	1_	29_	15_							<del></del>			·		

### Transportation Data Corporation

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Marshall Street south of Prentice Street City, State: Holliston, MA

NB											-0.4.1	A	> C A1	Not	
Start		Cars &	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axie	>6 Axl	<6 Axl	6 Axle	>6 Axl	Classe	Total
Time	Bikes	Trailers	Long	Buses	6 Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classe	Total
10/16/0					_	_	_	^	^	0	0	0	0	0	7
5	0	5	2	0	0	0	0	0	0	ő	ŏ	ŏ	ŏ	ŏ	3
01:00	0	2	1	0	0	0	0	Ö	0	ŏ	ŏ	ŏ	ŏ	ŏ	3
02:00	0	3	0	0	0	Ŏ	Ň	ŏ	ŏ	ŏ	ŏ	ő	Ó	0	0
03:00	0	0	0	0	Ô	Ŏ	ŏ	ŏ	ŏ	ŏ	Ö	0	0	0	2
04:00	0	2 7	2	0	ň	ŏ	ŏ	Ŏ	Ó	0	0	0	0	0	9
05:00	0	14	1	ŏ	1	ŏ	Ŏ	0	0	0	0	0	0	0	16
06:00 07:00	0	18	11	ŏ	i i	ŏ	0	0	0	0	0	0	0	0	30 32
08:00	1	16	14	ō	1	0	0	0	0	0	0	0	0		
09:00	0	45	21	0	1	0	0	1	0	0	0	0	0	0	68
	Ô	54	22	Ö	0	0	0	1	0	0	0	0	0	0	77
10:00	4	47	23	ő	2	Ó	0	0	0	0	0	0	0	0	76
11:00	-		18	ő	1	Ŏ	0	0	0	0	0	0	0	Ō	69
12 PM	2	48 42	8	Ö	ò	ŏ	ŏ	0	0	0	0	o o	0	0	50
13:00 14:00	ŏ	42 54	17	ŏ	ŏ	ō	Ö	0	0	0	0	0	0	0	71
	0	52	31	Ŏ	2	0	0	0	0	0	0	0	0	0 .	85
15:00	-	52 57	18	ŏ	7	ō	0	0	0	0	0	0	0	0	82
16:00	0	57 59	30	0	4	ŏ	ō	Ó	0	0	0	0	0	Ō	90
17:00	0	17	30 8	ő	ò	ŏ	ŏ	Ō	0	0	0	0	0	0	25
18:00 19:00	ő	7	1	ŏ	ŏ	ō	Ó	0	0	0	0	o o	0	0	8 12
20:00	0	11	1	ŏ	Ŏ	0	0	0	0	o o	0	0	0	0	9
21:00	ŏ	6	2	0	1	0	0	Ō	0	0	0	0	0	ŏ	5
22:00	ŏ	4	1	0	0	0	0	0	0	0	0	0	0	0	4
23:00	ŏ	3	0	0	1_	0	0	<u> </u>	0	0	0	- 0	0	0	833
Total	7	573	232	0	19	0	0	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	-
Percent	0.8%	68.8%	27.9%	0.0%	2.3%	0.0%	0.0%	U.Z76	0.076	0.078	0.070	0.0.0			
				····											10:00
MA	11:00	10:00	11:00		11:00			09:00							
Peak	4	54	23		2			1							77
Vol. PM					40.00										17:00
Peak	12:00	17:00	15:00		16:00										90
Vol.	2	59	31_		7										
												_		•	0042
Grand	12	1363	584	5	64	9	0	6	0	0	0	0	0	0	2043
Total					3.1%	0.4%	0.0%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
Percent	0.6%	66.7%	28.6%	0.2%	3.1%	U.470	0.076	0.070	5.57	2.2.77					

Transportation Data Corporation
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Cell (781) 316-4663 E: mperonel@comcast.net

Page 1 03464Aclass Site Code: 03464

Marshall Street south of Prentice Street

Start   Cars & 2 Axle   2 Axle   3 Axle   4 Axle   5 Axl   5 Axle   5 Axle   5 Axl   5 Axle   5 Axl   5 Axle   5 Axle   5 Axl   5 Axle	SB															
Time   Bikes   Trailers   Long   Blues   6 Tire   Single   Single   Double   Double   Double   Multi   Multi   Multi   Classe   Iotal	Start		Cars &	2 Axle		2 Axle	3 Axle	4 Axle		5 Axle	>6 AxI	<6 Axl	6 Axle	>6 AxI	Not	
10/14/0		Bikes			Buses	6 Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classe	<u>Total</u>
5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0													_	_		
03:00		0	0	0	0	0						-				0
02:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	01:00	0	1	0	0	0	0	_	-	0	0	o o	Ŏ		-	1
03:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0	0	0	0	0	0	Ŧ	~	0	0	0	0	-	V	0
04:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	03:00	0	0	0	0	0	0	·	Ÿ	0	Ü	Ü	v	•	Ň	ň
05:00 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0	0	0	· ·	0	0		•	Ü	Ň	0	ŏ	•	ň	1
06:00 0 4 7 2 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	05:00	0	0	1	0	0	0	•	•	•	•	0	•	•	•	,
07:00 0 15 14 2 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06:00	0	4	2	0	1	0	0	0	•	•	U		_	_	ລ່
08:00	07:00	0	15	14	2	1	0	0	0	0	-	0	•		•	
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10:00 0 8 5 5 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 28 11:00 0 15 10 2 1 0 0 0 0 0 0 0 0 0 0 0 0 0 25 12:PM 0 13 9 1 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 25 13:00 0 20 10 0 4 3 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		•			0	0	0	0	1	0	0	0	0		_	
11:00 0 15 10 2 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 25 12 PM 0 13 9 1 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 38 13:00 0 20 10 0 4 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 38 14:00 0 39 9 1 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 52 15:00 0 37 11 2 2 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		_			ŏ	ō	1	0	0	0	•	0	o o		-	
12 PM		-			2	1	0	0	0	_	-	0	0		•	
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18:00 0 41 14 0 1 1 1 0 1 0 0 0 0 0 0 0 0 0	17:00	•			ŏ	2	0	0	0	0	0	0	Ō	-	-	
19:00 0 29 8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		ŏ			Ó	1	1	0	1	0	0		o o	-	-	
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23:00   0   5   3   0   0   0   0   0   0   0   0   0		0	22	7		0	•	•	~	0	ŭ	•	ŭ	-	_	
Total Percent 0.0% 66.9% 27.0% 1.4% 3.2% 1.0% 0.0% 0.5% 0.0% 0.0% 0.0% 0.0% 0.0% 0		0		3		0				<u> </u>			<u> </u>			
AM 08:00 07:00 06:00 08:00 09:00 07:00 07:00 06:00 08:00 09:00 07:	Total															020
Peak         08:00         07:00         08:00	Percent	0.0%	66.9%	27.0%	1.4%	3.2%	1.0%	0.0%	0.5%	0.0%	0.0%	0.076	0.076	0.070	0.070	
Peak         Vol.         20         14         2         1         1         1         32           PM         16:00         16:00         15:00         13:00         13:00         16:00           Peak         80			08:00	07:00	07:00	06:00	08:00		09:00							07:00
Vol. 20 14 2 1 1 16:00 Peak 16:00 15:00 13:00 13:00 13:00 16						4	4		4							32
Peak 16:00 16:00 15:00 1			20	14		11	1.		1							
Peak 80			16:00	16:00	15:00	13:00	13:00		13:00							16:00
	Peak Vol.		58	21	2	4	3		1							80

# Transportation Data Corporation

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Marshall Street south of Prentice Street

SB					2 Aula	3 Axie	4 Axle	<5 Axl	5 Axle	>6 AxI	<6 Axi	6 Axie	>6 AxI	Not	
Start		Cars &	2 Axle	_	2 Axle		Single	Double	Double	Double	Multi	Multi	Multi	Classe	Total
Time	Bikes	Trailers	Long	Buses	6 Tire	Single	Sirigie	DOGDIE	Donnie	Dodbio	171014				
10/15/0		_		^	0	0	0	0	0	0	0	0	0	0	6
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08:00	0	15	4	•	2	1	ň	1	0	0	0	0	0	0	30
09:00	0	16	10	0	3	,	ő	0	ō	0	0	0	0	0	50
10:00	0	34	13	0	3	Ü	•	o	ŏ	Ō	0	0	0	0	55
11:00	0	38	15	0	1	1	0	-	0	Õ	ŏ	ō	0	0	43
12 PM	0	37	6	0	0	0	0	0	ŏ	ő	ŏ	ŏ	Ŏ	Ó	43
13:00	ō	31	12	0	0	0	0	•	•	ő	ŏ	ŏ	0	0	49
14:00	0	33	16	0	0	0	0	0	0	0	ŏ	ň	ŏ	ŏ	43
15:00	ō	33	10	0	0	0	0	0	•	_	Ô	0	0	Ó	29
16:00	ō	20	8	0	1	0	0	0	0	0	0	ň	ŏ	ŏ	29 35 28
17:00	Ö	21	13	0	1	0	0	0	0	Ö	ŏ	ň	ŏ	ŏ	28
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22:00	0	15	1	0	0	0	0	0	ŏ	-	ō	Ó	0	0	16
23:00	0	13	3	<u> 0</u>	- 0	<u>0</u> 2	<u>0</u>	2	ŏ		0	. 0	0	0	533
Total	0	392	126	0	11	0.4%	0.0%	0.4%	0.0%		0.0%	0.0%	0.0%	0.0%	
Percent	0.0%	73.5%	23.6%	0.0%	2.1%	U.476	0.078	0.470	0.070						
AM		11:00	11:00		10:00	09:00		07:00							11:00
Peak						4		1							55
Vol. PM		38	15		3			<u>-</u>							14:00
PM Peak		12:00	14:00		16:00										49
Vol.		37	16		1										

### Transportation Data Corporation

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Marshall Street south of Prentice Street

SB									C Auto	> C AI	<6 AxI	6 Axle	>6 AxI	Not	
Start		Cars &	2 Axle		2 Axle	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl		Multi	Multi	Classe	Total
Time	Bikes	Trailers	Long	Buses	6 Tire	Single	Single	Double	Double	Double	Multi	wun	Wichti	Classe	Total
10/16/0								^	^	0	0	0	0	0	7
5	0	5	1	0	1	0	0	0	0	ŏ	ŏ	ŏ	ŏ	Ŏ	1
01:00	0	1	0	0	0	0	0	0	ŏ	ŏ	ŏ	ŏ	Ö	Ó	3
02:00	0	3	0	0	0	0	ŏ	ŏ	ő	ŏ	ŏ	Ó	0	0	1
03:00	0	0	1	0	0	0	ő	ŏ	ŏ	ŏ	Ō	0	0	0	1
04:00	0	1	0	0	V	ŏ	ŏ	ŏ	Ŏ	0	0	0	0	Ō	0
05:00	0	0	0	0	ň	ŏ	ŏ	ŏ	Ó	0	0	0	0	Ō	1
06:00	0	1	0	0	Ŏ	ŏ	ŏ	ŏ	0	0	0	0	0	o o	6
07:00	0	6	<del>-</del>	0	ň	Ô	Ö	0	0	0	0	0	0	1	35
08:00	0	26	8	-	2	Ŏ	Ö	ō	Ó	0	0	0	0	0	31
09:00	1	19	9	0	0	Ö	ŏ	ŏ	ŏ	Ŏ	Ó	0	0	0	31
10:00	0	26	5	-		0	ŏ	Õ	0	0	0	0	0	0	55
11:00	1	34	18	0	2		ŏ	1	ŏ	ò	0	0	0	0	92
12 PM	1	66	22	0	2	0	ŏ	Ö	ŏ	ŏ	Ō	0	0	0	81
13:00	0	59	21	0	1	ŏ	ŏ	ŏ	ŏ	ō	0	0	0	0	85
14:00	0	63	21	-		ő	ő	ō	0	0	0	0	0	0	100
15:00	1	73	25	0	1	ő	0	ŏ	ŏ	Ō	0	0	0	O.	61
16:00	1	39	20	0	4	ŏ	ŏ	ŏ	Ō	0	0	0	0	0	41
17:00	1	31	8 9	ŏ	1	ŏ	ō	Ö	0	0	0	0	0	0	34 17
18:00	Ŏ	24 10	6	ŏ	i	Ö	Ó	0	0	0	0	0	0	0	19
19:00 20:00	0	13	6	ŏ	ó	Ō	0	0	0	0	0	0	0	0	8
20:00	ŏ	6	ĭ	ō	1	0	0	o.	0	0	0	0	ŏ	ŏ	5
22:00	ŏ	3	i	Ò	1	0	0	0	0	Ŏ	0	0	ŏ	ŏ	7
23:00	ŏ	5	2	0	0	0	0	0	Ŏ	0	0	0	ŏ	1	722
Total	6	514	184	0	16	0	0	1	0.0%	0.0%	0.0%	0.0%	0.0%	0.1%	
Percent	0.8%	71.2%	25.5%	0.0%	2.2%	0.0%	0.0%	0.1%	0.0%	0.076	0.070	0.070	•,•		
, •														08:00	11:00
AM	09:00	11:00	11:00		09:00									00.00	
Peak	03.00				2									1_	55_
Vol.	1	34	18					40.00							15:00
PM	12:00	15:00	15:00		12:00			12:00							100
Peak		73	25		2			1							100
Vol.	1	(3													
Grand	_		4-0	_	47	8	c	. 6		0	0	0	0	1	1881
Total	6	1325	479	9							0.0%	0.0%	0.0%	0.1%	
Percent	0.3%	70.4%	25.5%	0.5%	2.5%	0.4%	0.0%	0.3%	0.0%	0.0%	0.070	V.V70	0.070	270	
, 0.00111															

# Capacity Analyses

Cedar Ridge EstatesTraffic StudyHolliston, MAOctober, 2005

			Site Inform	nation			]
eneral Information	· · · · · · · · · · · · · · · · · · ·			Hation	Marshall St.	@ Prentice	,
Analyst	JBB		Intersection		St.		<u> </u>
Agency/Co.	Coler & Co	lantonio	Jurisdiction		Holliston		•
Date Performed	9/26/05		Analysis Yea	ar	2005 AM Exi	sting	
Analysis Time Period	AM Peak (	7:00-8:00am)					
	DIs Hour E	xisting Conditio	ns				]
Project Description AM I	Peak Hour E	xisting Conditio	North/South	Street: Mar.	shall Street		
ast/West Street: Prentic	East-Mest		Study Period	i (hrs): 0.25			<u> </u>
ntersection Orientation:							
/ehicle Volumes and	<u>l Adjustm</u>	ents			Westbound		_ *
Najor Street		Eastbound	3	4	5	6	]
Movement	1	2 T	R	L	Т	R	
	<u> </u>	211	7	15	218		_
/olume	0.05	0.85	0.85	0.85	0.85	0.85	4
Peak-Hour Factor, PHF	0.85 0	248	8	17	256	0	4
Hourly Flow Rate, HFR	2			2			
Percent Heavy Vehicles			i	ivided			_
Median Type			1 0			0	_]
RT Channelized		1	0	0	1	0	
Lanes	0	1		. 1			1
			70	TT	1	<u> </u>	
			TO	I T		{	
			TO	I T		<u> </u>	
			TO			<u> </u>	
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Sanaral Information	<b>\</b>		Site In	form	atic	n				
eneral Information	1				-		Marsha	all St.	@ Prer	tice
\nalyst	JBB		Intersec	ction			St.	.,,		
\gency/Co.	Coler & C	olantonio	Jurisdic	tion			Hollisto	n		
Date Performed	9/26/05		Analysi	s Yea	r		2005 F	M Ex	isting	
Analysis Time Period	PM Peak	(4:45-5:45pm)	_				<u> </u>			
roject Description PN	1 Peak Hour E	xisting Conditio	ns							
ast/West Street: Prent	ice Street		North/S			t: <i>Marsi</i>	hall Stre	et		
ntersection Orientation:			Study P	eriod	(hrs)	: 0.25				
ehicle Volumes ar	nd Adiustm	ents								
lajor Street		Eastbound					Westb			
Novement	1	2	3			4	5			6
	ال	Т	R			<u> </u>	7		<del> </del>	R
/olume		214	18			64	25		-	.85
eak-Hour Factor, PHF	0.85	0.85	0.85			).85 75	0.8 29			.05 0
lourly Flow Rate, HFR	0	251	21			75 2	29			
Percent Heavy Vehicles	2	<u> </u>		Undiv	idad	4	<u> </u>			
Median Type				UriaiV	iueu		T		1	0
RT Channelized			0		,	0	1		-	0
anes	0	1	0 TD			<u>U</u> LT	<u> </u>			
Configuration			TR			LI	0		-	
Jpstream Signal		0	<u> </u>				South			
Minor Street		Northbound			,,	10	50uiii		1	12
Movement	7	8	9			L	-			R
	L	T	R			<u> </u>				
Volume	20	0.05	41 0.85			0.85	0.8	35	1 7	.85
Peak-Hour Factor, PHF	0.85	0.85	48		· · · · ·	0	0.0			0
Hourly Flow Rate, HFR	23	1 0	$\frac{1}{0}$			0		)		0
Percent Heavy Vehicles	1	0	<u> </u>				(	)		
Percent Grade (%)		N					1 1	7		
Flared Approach								<u> </u>	1	
Storage	<u> </u>	0					1			0
RT Channelized		<del></del>	0			0	<del> </del>	)		0
Lanes	0	0	0			U	<del>                                     </del>	<u> </u>	-	<u> </u>
Configuration		LR	<u> </u>							
Delay, Queue Length,	and Level of	Service		1 - 1-1-1-				Sau	thbound	4
Approach	Eastbound	Westbound		lorthb			10		11	12
Movement	1	4	7	8		9	10	$\dashv$	1 1	'~
Lane Configuration		LT		LF				-		<del> </del>
v (vph)		<i>75</i>		71						ļ
C (m) (vph)		1291		58.	2					
v/c		0.06		0.1	2					
95% queue length		0.18		0.4	1					
Control Delay		8.0		12.	.0					
		A		В						
LOS				12.		<u> </u>				
Approach Delay			<u> </u>	Б						

	TWO-	WAY STOP (							
General Information	1		Site In	form	atio	on			
Analyst	JBB		Intersec	etion			Marshall S	St. @ Pro	entice
Agency/Co.	Coler & C	olantonio					St. Holliston		
Date Performed	9/26/05		Jurisdic				2005 SAT	Evicting	y
Analysis Time Period		(11:00am-	Analysi	s yea	r		2005 SAT	LAISHING	!
	12:00pm)	= :							
		Existing Conditi	ONS North/C	outh C	2troo	t. March	nall Street		
ast/West Street: Pren			Study P				ian Otreot	<u> </u>	
ntersection Orientation:			Study	enou	(110)	. 0.2.0			
/ehicle Volumes a	nd Adjustm	ents					Westboun	d	
lajor Street		Eastbound		⊦		1	wesibour 5	<u> </u>	6
Movement	11	2	3			4	T		R
	L	T 457	R 25			99	146		
/olume	0.05	157 0.85	25 0.85			99 0.85	0.85		0.85
Peak-Hour Factor, PHF	0.85 0	184	29	-+		116	171		0
Hourly Flow Rate, HFR						2			*-
Percent Heavy Vehicles	<del> </del>		1	Undiv	ided				
Median Type			0						0
RT Channelized	0	1	0			0	1		0
anes			TR	<del></del>		LT			
Configuration		0	771				0		
Jpstream Signal	<u> </u>	Northbound	<u> </u>				Southbou	nd	
Minor Street	ļ	Normbound 8	9			10	11		12
Movement	7	T	R			L	Т		R
			106						
Volume	24 0.85	0.85	0.85			0.85	0.85		0.85
Peak-Hour Factor, PHF	28	0.00	124			0	0		0
Hourly Flow Rate, HFR Percent Heavy Vehicles	_1	0	0			0	0		0
Percent Grade (%)	<u> </u>	0	1				0		
		Ň	1				N		
Flared Approach		1 0					0		
Storage		-	0				1		0
RT Channelized		<del>                                     </del>	0			0	0		0
Lanes	0	0	<i>-</i>			<u> </u>	<del>                                     </del>		<del>-</del>
Configuration	<u> </u>	LR							
Delay, Queue Length,		Service		t		J	T 6.	outhbou	nd
Approach	Eastbound	Westbound		Iorthb				11	12
Movement	1	4	7	8		9	10	3 1	12
Lane Configuration		LT		LF					_
v (vph)		116		15.			<u> </u>	<u> </u>	
C (m) (vph)		1357		71	6			<u></u>	
v/c		0.09		0.2	?1				
95% queue length		0.28	<u> </u>	0.8	30				<u> </u>
Control Delay		7.9		11.					
		A		B					
LOS				11.				<u></u>	
Approach Delay				E					
Approach LOS		**	1		,				

	TWO-	WAY STOP (										
General Informatio	n		Site In	forma	atio	n						
Analyst	JBB				Intersection				Marshall St. @ Prentice St.			
Agency/Co.	Coler & C	olantonio	Jurisdiction				Holliston					
Date Performed	9/27/05			Analysis Year			2010 AM No-Build					
Analysis Time Period	AM Peak	(7:00-8:00am)	<b>-    </b>									
Project Description A	M Peak Hour I	No-Build Condition	ons									
ast/West Street: Pren			North/S				hall Street					
Intersection Orientation: East-West			Study Period (hrs): 0.25									
/ehicle Volumes a		ents										
Major Street		Eastbound					Westbour	nd				
Movement	1	2	3			4	5		6			
	L	Ī	R			L	T		R			
/olume		251	8			18	259		0.05			
Peak-Hour Factor, PHF		0.85	0.85			.85	0.85		0.85			
Hourly Flow Rate, HFR	0	295	9			21	304		0			
Percent Heavy Vehicles	2			110000		2						
Median Type				Undivid	ied		T		0			
RT Channelized			0						0			
anes .	0	1	0	0		1		U				
Configuration			TR		ı	T						
Jpstream Signal		0	<u> </u>				0	<u> </u>				
Minor Street		Northbound					Southbou	ind	10			
Movement	7	8	9			11		12				
	L	T	R			L	Т		R			
Volume	30		80		0.05		0.05		0.85			
Peak-Hour Factor, PHF		0.85	0.85			0.85 0		0.05				
Hourly Flow Rate, HFR		0	94			0	0		0			
Percent Heavy Vehicle:	s 2	0	0			U	0					
Percent Grade (%)		0					T N					
Flared Approach		N										
Storage		0					0					
RT Channelized			0						0			
Lanes	0	0	0			0	0		0 .			
Configuration		LR	<u> </u>									
Delay, Queue Length,	and Level of	Service										
Approach	Eastbound	Westbound	٨	lorthbo	und			outhbou				
Movement	1	4	7	8	$ \_                                   $	9	10	11	12			
Lane Configuration		LT		LR								
v (vph)		21		129								
		1257		620				T T				
C (m) (vph)		0.02		0.21			1					
v/c		0.02		0.78				t				
95% queue length			<u> </u>	12.3			-	<del> </del>	_			
Control Delay		7.9			<u>'</u>			-				
LOS		Α		B		<del>,</del>		<u> </u>				
Approach Delay			12.3									
Approach LOS			1	В								

General Information			Site Inform	nation				
Analyst <i>JBB</i>			Intersection		Marshall St. @ Prentice			
Agency/Co. Coler & Colantonio			Jurisdiction		Holliston	St.		
Date Performed	9/27/05		Analysis Ye	ar		2010 PM No-Build		
Analysis Time Period PM Peak (4:45-5:45pm)		Thirdiysis re	a:	20707117710 20110				
	De etal Llevin N	o-Build Conditio	IL					
Project Description PM I East/West Street: Prentic		o-Baila Conaine	North/South	Street: Mars	hall Street			
ntersection Orientation:				d (hrs): 0.25				
Vehicle Volumes and	Aajustm	Eastbound			Westbound			
Major Street	1	Lastiduria 2	3	4	5	6		
Movement	L	T	R	L	Т	R		
Volume	<u></u>	254	21	<i>7</i> 6	298			
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85		
Hourly Flow Rate, HFR	0	298	24	89	350	0		
Percent Heavy Vehicles	2			2				
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street		Northbound			Southbound			
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	24		49	2.55	0.05	0.85		
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85 0	0.83		
Hourly Flow Rate, HFR	28	0	<i>57</i>	0 0	0	1 0		
Percent Heavy Vehicles	2	0	0	U				
Percent Grade (%)		0	1	0				
Flared Approach		N			N	-		
Storage		0			0	<del> </del>		
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR		<u> </u>				

	TWO-WAY STOP CONTROL SUMMARY								
	General Information		Site Information						
	Analyst	JBB	Intersection	Marshall St. @ Prentice St.	į				
• 6	Agency/Co.	Coler & Colantonio	I	Ualliston	• .				
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Seneral Information	<u> </u>		Site In	Torn	iatic	on	1.4	24 @ D.			
Analyst	JBB	Intersection			Marshall St. @ Prentice St.						
Agency/Co.	Coler & Colantonio		Jurisdio	Jurisdiction			Holliston				
Date Performed	9/26/05		Analysis Year			2010 AM	2010 AM Build				
Analysis Time Period	AM Peak	(7:00-8:00am)	╝								
Project Description AA	A Peak Hour E	Build Conditions			-						
ast/West Street: Prent							hall Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25							
/ehicle Volumes ar	nd Adiustm	ents									
lajor Street		Eastbound					Westbour	ıd			
Novement	1	2	3			4	5		6		
	L	Т	R			<u>L</u>	T		R		
/olume		251	12			25	259 0.85		0.85		
Peak-Hour Factor, PHF	0.85	0.85	0.85			).85 20	0.85 304		0.65		
Hourly Flow Rate, HFR	0	295	14			29 2	304				
Percent Heavy Vehicles	2			Undiv	اندام		1				
Median Type	<u> </u>			Unaiv	iuea			T.	0		
RT Channelized			0			0	1		0		
.anes	0 :	1 1	<u> </u>			LT	<del> </del>				
Configuration			TR			LI	0				
Jpstream Signal	<u> </u>	0	<u> </u>				Southbou	nd			
Minor Street		Northbound				10	11	iiu .	12		
Movement	7	8	9				<del>                                     </del>		R		
	L	T	R 111			<u> </u>	<u> </u>		•••		
Volume	52	0.85	0.85	0.85		0.85		0.85			
Peak-Hour Factor, PHF	0.85 61	0.85	130			0		0			
Hourly Flow Rate, HFR		0	0			0	0		0		
Percent Heavy Vehicles	1	0	<u> </u>				0				
Percent Grade (%)		TN	T				N N				
Flared Approach	<u> </u>	0	<u> </u>			·····	0				
Storage			0						0		
RT Channelized			0			0	1 0		0		
Lanes	0	0 LR	U			V	+ -				
Configuration			<u> </u>								
Delay, Queue Length,	and Level of	Service		ماطاسما	01100		1 6	outhbou	nd		
Approach	Eastbound	Westbound		lorthb		ı 9	10	11	12		
Movement	1	4	7	8		<del>"</del>	1 10	<b></b>			
Lane Configuration		LT		LF					_		
v (vph)		29		19				<u> </u>			
C (m) (vph)		1252		59							
v/c		0.02		0.3	32						
95% queue length		0.07		1.3	39						
Control Delay		7.9		13.	9						
LOS		Α		В	}						
Approach Delay			13.9								
Approach LOS				Б							

	TWO-	WAY STOP (										
General Information	on		Site In	form	atic	on			<u> </u>			
Analyst	JBB				Intersection			Marshall St. @ Prentice St.				
Ageńcy/Co.	Coler & C	olantonio	Jurisdiction				Holliston					
Date Performed	9/26/05			Analysis Year			2010 PM Build					
Analysis Time Period	PM Peak	(4:45-5:45pm)	<b>╝</b>									
Project Description F	M Peak Hour E	Build Conditions										
ast/West Street: Pre							hall Street					
Intersection Orientation: East-West				Study Period (hrs): 0.25								
/ehicle Volumes a		ents										
Major Street	The Aujubin	Eastbound					Westbour	าต์				
Movement	1 1	2	3			4	5		6			
NOVOINOIR	L.	Τ	R			L	Т		R			
/olume		254	42			105	298		0.00			
Peak-Hour Factor, PHF		0.85	0.85			0.85	0.85		0.85			
Hourly Flow Rate, HFR		298	49			123	350	_	0			
Percent Heavy Vehicle				<u></u>		2						
Median Type				Undiv	raed		T		2 <b>0</b>			
RT Channelized			0									
anes	. 0	1	0		0		1		<u>0</u>			
Configuration :			TR			LT		_				
Jpstream Signal		0	<u> </u>				0					
Minor Street		Northbound					Southbou	nd	40			
Movement	7	8	9			10	11		12			
	L	T	R			L	Т		R			
Volume	34		56		0.05		0.85		0.85			
Peak-Hour Factor, PHI		0.85	0.85	0.85 0		0.85		0.00				
Hourly Flow Rate, HFF		0	65			0	0		<del>-0</del>			
Percent Heavy Vehicle	es <u>2</u>	0	0			U .	0					
Percent Grade (%)		0	· · · · · · · · · · · · · · · · · · ·				N					
Flared Approach		N						<del> </del>				
Storage		0					0		0			
RT Channelized			0				<del> </del>		0			
Lanes	0	0	0			0	0		U			
Configuration		LR										
Delay, Queue Length	, and Level of	Service										
Approach	Eastbound	Westbound		Iorthb				outhbou				
Movement	1	4	7	8		9	10	11	12			
Lane Configuration		LT		L.F	?							
v (vph)		123		10	4							
C (m) (vph)		1212		44	5 .							
v/c		0.10		0.2	23							
95% queue length		0.34		0.9								
		8.3		15.		<b></b>						
Control Delay				, o.				<del>                                     </del>				
LOS		Α				<u>L</u>	+	<u>. L </u>				
Approach Delay			15.5									
Approach LOS			<u> </u>	C	<u></u>							

	1 440-	WAY STOP	CONTIN	<i></i>	O IVII	VIZALLE			
General Information			Site In	iforn	natio	on			
Analyst	JBB		Interse	otion			Marshall 3	St. @ F	Prentice
Agency/Co.	Coler & C	olantonio					St.		
Date Performed	9/26/05		Jurisdio				Holliston	F (02.4)	
Analysis Time Period	SAT Peak 12:00pm)	c (11:00am-	Analysi	is Yea	r		2010 SAT	Bulla	
Project Description SA	T Peak Hour	<b>Build Condition</b> :	S						
ast/West Street: Prenti							nall Street		
ntersection Orientation:	East-West		Study F	eriod	(hrs	): <i>0.25</i>			
/ehicle Volumes an	d Adjustm	nents							
Najor Street		Eastbound					Westbour	nd	
/lovement	1	2	3			4	5		6
	L	T	R			L	T 470		R
/olume		186	46			141	173		0.05
eak-Hour Factor, PHF	0.85	0.85	0.85			).85 165	0.85 203		0.85 0
lourly Flow Rate, HFR	0	218	54			165	203		
Percent Heavy Vehicles	2			/ / / / / / /	ام د اداد	2	**		
Median Type				Undiv	riaea			<del></del>	0
RT Channelized			0						0
anes.	0	1	0	`		0	1		U
Configuration			TR	;		LT	0		
Jpstream Signal		0	1						
Minor Street		Northbound					Southbound		12
Movement	7	8	9			10	11		
	L	Т	R		<u></u>	<u>L</u>	Т		R
/olume	43		144			0.05	0.05	0.85 0.	
Peak-Hour Factor, PHF	0.85	0.85	0.85			0.85 0	0.85		0.85 0
Hourly Flow Rate, HFR	50	0	169			0	0		0
Percent Heavy Vehicles	2	0	0			U	0		
Percent Grade (%)		0	T						
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0					ļ_	0
Lanes	0	0	0			0	0		0
Configuration		LR			<u> </u>				
Delay, Queue Length, a	nd Level of	Service							
Approach	Eastbound	Westbound	N	Iorthb	ounc		S	outhbo	
Movement	1	4	7	8		9	10	11	1
Lane Configuration		LT		LF	₹				
v (vph)		165		21:	9				
		1291		59					
C (m) (vph)		0.13		0.3					
v/c		0.13		1.6				<del>                                     </del>	_
95% queue length								-	
Control Delay	· · · · · · · · · · · · · · · · · · ·	8.2	<u> </u>	14.				<del> </del>	
LOS		A		B		<u></u>	-	<u></u>	
Approach Delay				14.					
Approach LOS			1	В	}		1		

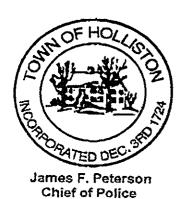
	TWO	WAY STOP	CONTRO	OL SU	JMI	VIARY			
General Informatio	n		Site Ir	nform	atio	on			
Analyst	JBB		Interse	ction			Marshall St.	St. @ Pr	entice
Agency/Co.	Coler & C	olantonio	Jurisdie	Jurisdiction			Holliston		
Date Performed	10/21/05	(4.45 5.45 pm)	Analys	is Yeaı	r		2010 PM	Build w/	Soccer
Analysis Time Period	РМ Реак	(4:45-5:45pm)	┦[						
		Build Conditions							
ast/West Street: Prer							nall Street		
ntersection Orientation:	East-West		Study F	erioa	(nrs)	: 0.25			
Vehicle Volumes a	nd Adjustn						141 3		
Major Street		Eastbound					Westbour	nd	
Movement	1	2	3			4	5 T		6 R
	<u> </u>	T	R 65			<u>L</u> 243	298		П
/olume	1 000	254	65 0.85			243 2.85	0.85		0.85
Peak-Hour Factor, PHF		0.85 298	76			285	350		0.00
Hourly Flow Rate, HFR	0 2	298	70	-		2			
Percent Heavy Vehicles			<u> 1</u>	Undivi	ded				
Median Type	<del> </del>		0	SHOW				0	
RT Channelized	0	1	0			0	1		0
anes	1		TR			LT			
Configuration	:	0	777				0		
Jpstream Signal							Southbound		
Minor Street	<del></del>	Northbound 8	9			10	11	1	12
Movement	7	T	R			L	Т		R
	103		148			<u> </u>			
Volume Peak-Hour Factor, PHF		0.85	0.85	_	(	0.85	0.85	0.85 0.	
Hourly Flow Rate, HFR		0.00	174			0	0		0
Percent Heavy Vehicles		0	0			0	0		0
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	1 0	0	0		·	0	0		0
Configuration		LR							
Delay, Queue Length,	and I evel of								
Approach	Eastbound	Westbound	N	lorthbo	und		s	outhbour	nd
Movement	1	4	7	8		9	10	11	12
Lane Configuration	•	LT	· · · · · · · · · · · · · · · · · · ·	LR	一				
v (vph)		285		295		······································	<del>                                     </del>		
		1184		272				1	1
C (m) (vph)		0.24		1.08					
V/C				12.0			<del>                                     </del>		1
95% queue length		0.94		<u> </u>					<del>- </del>
Control Delay		9.0		119.	<del>"</del>		<u> </u>	<del>                                     </del>	
LOS		Α		F	لــ			]	
Approach Delay				119.	9				
Approach LOS			l	F			<u> </u>		

		WAY STOP C	Site In		ati∩							
ieneral Informatio	<u>n</u>		Site iiii	01111	atio	1 .	Marshall S	i @ Pr	entic	e		
Analyst	JBB		Intersec	tion			St.	• • • •	V. 1670			
\gency/Co.	Coler & Co	olantonio	Jurisdic				Holliston					
Date Performed	10/21/05		Analysis	Year	1		2010 Build	w/Soc	cer			
Analysis Time Period	Weekend	mid-day Peak										
roject Description W	eekend mid-da	y Peak Hour Bu	uild Conditi	ons								
ast/West Street: Pren	tice Street		Morninge	Julii 3			all Street					
ntersection Orientation:			Study Pe	eriod	(hrs):	0.25						
/ehicle Volumes a		ents										
Major Street	1	Eastbound					Westboun	d				
Najor Street Novement	1	2	3			4	5		6			
NO 40110114	L	T	R			<u>L</u>	T 470		R			
/olume		186	69			79	173		0.85	<del></del>		
Peak-Hour Factor, PHF		0.85	0.85			.85	0.85 203		0.00			
lourly Flow Rate, HFR	0	218	81			28 2	203	_				
Percent Heavy Vehicles	2			د المال		4						
Median Type				Undiv,	iuea.			0				
RT Channelized			0			0	1		0			
anes	0	1	70			LT						
Configuration			TR			L /	0					
Upstream Signal		0	<u> </u>			<u> </u>	Southbou	nd nd				
Minor Street		Northbound	· .			10	11	na (	12	>		
Movement	7	8	9 R			L	T		F			
	L.	T	236	L		<u> </u>	•					
Volume	112	0.05	0.85			).85	0.85	_	0.8	5		
Peak-Hour Factor, PHF		0.85	277			0	0.00		0		0	
Hourly Flow Rate, HFR		1 0	0			0	0		0			
Percent Heavy Vehicle	5 2	0					0					
Percent Grade (%)		T N	T T				N					
Flared Approach		0	<u> </u>				0		***************************************			
Storage			0						0	)		
RT Channelized		0	0			0	0		0	)		
Lanes	0	LR	1 - <del>'</del>			<u> </u>	<del>                                     </del>					
Configuration												
Delay, Queue Length	and Level of	Service	T N	lorthb	ound		l s	outhbou	ınd			
Approach	Eastbound	Westbound	7	8		9	10	11	Т	12		
Movement	11	4		LF			<del>                                     </del>	<del>                                     </del>	十			
Lane Configuration		LT							十	·		
v (vph)		328		40				<del> </del>	$\dashv$			
C (m) (vph)		1262		36				<del> </del>				
v/c		0.26		1.1								
95% queue length		1.04		15.			_	<b></b>				
Control Delay		8.9		120								
LOS		Α		F	-			<u> </u>				
Approach Delay				120	).1							
Approach LOS				F	-							

## Crash Rate Worksheets

## CRASH RATE WORKSHEET

CITY/TOWN: 1	HOLLISTON, UNSIGNA	LIZED:	ERSECTION		TE: S	EPT, 2005	MHD USE ONLY Source #	1
MAJOR STREET:	PRENTICE S	TREET					ST# []	•
MINOR STREET(S):	MARSHALL:	STREET					ST#	;
•							ST#	
·							ST#	
INTERSECTION	North		2	1			REF#	
DIAGRAM (Label Approaches)	-							
(Laurent pro-	1	PRENTICE			STREET	2		•
				L STREET				
		·	Peak Hou	r Volumes				ia V
APPROACH:	1	2	3	4	Total Entering			
DIRECTION:	EB	WB	NB	SB	Vehicles			*
VOLUMES (AM(PM)):	232	315	61	0	608			
		1			29			
		_						
17								



Tel. 508-429-1212 Fax. 508-429-0611

# Holliston Police Department 532 Washington Street Holliston, Massachusetts 01746

FAX TRANSA	AISSION	
TO:	Jeff Bandini - Colera Colantonio	
	1-781-982-5490	
FROM:	Judi Johnson	
RE:	accident collectur data	
		• •
PAGES:		•
DATE:	10/5/05	<b>.</b>
COMMENTS:	Ruervied data for intersection	
	_ of Prentice & Marshall Sts.	
	2001 - 2005 - as requested	
		7 V 5
Confidentiality of I	Documents	
	attachments to this facsimile transmission are intended only for the use of the person or entity to which it is ontain information that is privileged, confidential, or exempt from disclosure under applicable law. If the reader of the intended recipient, you are hereby notified that any discontinuous discontin	
<b></b>		
	<b>.</b> .	

10/05/2005 10:52

15084290611 HOLLISTON POLICE DEP HOLLISTON Police Department From: 01/01/2001 Thru: 10/05/2005

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Prentice + Marshall Sts.

## Accident Statistics By Time of Day

		SUN	WON	TUE	WED	THR	FRI	SAT	TOTALS
1	AM	0	٥	<b>O</b>	0	0	0	0	0
2	AM	0	0	0	O	0	0	0	0
3	AM	0	0	0	0	0	0	0	0
4	AM	0	0	0	0	0	0	0	0
5	AM	0	0	٥	0	0	٥	0	0
6	AM	0	0	0	0	0	0	0	0
7	AM	0	0	0	0	0	0	0	0
8	AM	0	0	0	0	0	0	0	0
9	MA	Q	0	0	0	0	0	O	0
10	AM	0	<b>O</b>	0	0	0	0	0	0
11	AM	0	0	0	0	O	0	0	0
12	PM	0	0	0	0	0	0	0	0
1	PM	O	0	0	0	0	0	0	0
2	PM	0	0	0	0	0	0	0 .	٥
3	PM	0	0	0	0	0	0	0	0
4	PM	0	0	΄0	0	0	0	0	0
5	PM	0	O	0	0	O.	0	0	0
6	PM	0	0	0	0	0	Ó	٥	0
7	PM	0	0	0	0	0	0	0	Ġ.
8	PM	0	0	0	0	0	0	0	0
9	PM	0	0	0	0	0	0	0	0
10	PM	0	0	0	0	0	0	0	0
11	PM	0	0	0	0	0	0	0	0
12	AM	0	0	0	0	0	٥	0	0
T	OTALS		0					0	

## Accident Particulars

	Occurrence(s)	Percentage
Average posted speed at the accident scene		о мрн
Occurred at On-ramps	0	0.0
Occurred at Off-ramps	0	0.0
Occurred at an intersection	0	0.0
Occurred at a rotary	Ô	0.0
Occurred on a one lane road/highway	0	0.0
Occurred on a two lane road/highway	0	0.0
Occurred on a three lane road/highway	0	0.0
Occurred on a four lane road/highway	0	0.0
Occurred on other number of lanes	0	0.0
Involved OUI violation(s)	0	0.0
Photos were taken	0	0.0
Measurements were taken	Ò	0.0
Investigation took place	0	0.0
Involved Injuries	Ö	0.0
Involved Fatalities	0	0.0

10/05/2005 10:52

15084290611 HOLLISTON POLICE DEP Holliston Police Department From: 01/01/2001 Thru: 10/05/2005

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Przntice + Marshall Sts.

Age and Sex Breakdown of Operators

	<u> &lt; 19</u>	19-21	22-25	26-35	36-45	46~60	> 60	TOTALS
Male	0	0	0	0	0	0		
Female	٥	o	٥	0	0	0	p	0
TOTALS	0	0	0	0	0		0	

	Occurrence (s)	Percentage
Number of out of state operators Number of operators who were cited	0 0	0.0

## MHD Seasonal and Growth Tables

ITALICS = ESTIMATED DATA

# SECTION I - CONTINUOUS COUNTING STATION MONTHLY AVERAGE DAILY TRAFFIC

DEC YEAR 66,479 81,547					80,629 89,822	DEC	נו	71,002	-25%	53,118	, 44% 10%	76,482	%9	81,000	-2%	77.058	2
NOV 76.811	-5%	74,936	3%	77,119	93,591	) ON	2	64,401	28%	82,301	%0	82,654	-5%	81.336	%0	81 724	
OCT 81.951	1%	82,545	%	83,089	102,987	L	5	76,150	11%	84.491	4%	88.210	2%	90.039	-1%	00000	20,50
SEP 70.200	15%	80,944	<u>%</u>	82,057	102,902	010	r L L	78,501	13%	88,608	%0	88.447	2%	90.471	.1%	2000	20000
AUG 95.805	-10%	86,272	27%	109,531	107,867	2	AUG	74.847	26%	04 150	4%	08 264	,	07.641	31,041	20000	90,020
JUL	%61-	80,533	13%	90,836	106,876	:	JOC	85.644	25%	81 62E	%V+	00 860	36,26	00000	85,007	84.	97,448
JUN 345 FG	97,716 -12%	85,800	13%	96,649	104,000	:	<u> </u>	87.081	, se	24 63 63	000'16	0,000,00	34,48	% ? ?	91,628	%4%	95,000
F RTE.I-90 MAY	90,432	84.219	14%	96,102	99,532	ORD T.L.	VAM	74 260	) 1 1 1 1 1	%1.70 11.70	73,738	84.	84,451	%6	88,517	%0	88,240
5 - SOUTH O	82,638	71.889	%9	75,936	81,163	5 - AT MILFO	004	100400	70,122	827	77,620	%9	82,000	%7	83,664	%0	83,558
ON - RTE.I-49 MAR	73,842	-4% 71 026	701.	67,965	67,620	ON - RTE.I-49	0 0 0	UMMI 000	58,40p	%L	75,695	-5%	74,235	2%	78,247	<del>"</del>	78,796
6 - HOPKINT( FEB	72,581	-2%	) ) )	-3% 64,515	63,593	STATION 4797 - HOPKINTON - RTE.I-495 - AT MILFORD T.L.	(	1 1 1	67,104	-14%	57,741	30%	74,960	3%	76,893	-2%	73,249
STATION 4796 - HOPKINTON - RTE.I-495 - SOUTH OF RTE.I-90 JAN FEB MAR	70,503	-1%	70,007	17% 82,198	67,103	STATION 479		NAC.	55,656	%	55,512	24%	68,743	%	73,973	1%	74.932
 ∺	66	ć	3	10	93		!	X E	66		8		5		05		03

## ITE Trip Generation



PROJECT NO

CALCBY JBB

SUBJECT TICIP

9/23/05

GENERATION CHECKED BY

LOCATION HOLLISTON, MA DATE

## TRIP GENERATION

200 RESIDENTIAL TOWNHOUSES

LUC 230: RESIDENTIAL CONDOMINIUM / TOWNHOUSE

WEEKDAY

Ln (T): 0.85 Ln (200) + 2.55

= 7.05

T = 1,158 vpd

50% ENTER: 579 upd

50% EXIT: 579 upd

AM PEAK HOUR

Ln (T) = 0.80 Ln (200) + 0.26

: 4.50

T = 90 vph

17% ENTER: 15 uph

83% EXIT 75 VPL

PM PEAK HOUR

Ln (T): 0.82 Ln(200) +0.32

= 4.66

T = 106 vph

67% ENTER = 71 vph

33% EXIT. 35 Uph

SATURDAY

T= (3.62)(200)+427.93

T= 1,152 ypd

50% ENTER = 576 vpd 50% EXIT: 576 VPd

SATURDAY PEAK HOUR

T= (0,29)(200) +42.63

T= 101 vph

54% ENTER = 55 VPh 46% EXIT = 46 VPh

> OF. SHEETS SHEET

## Marshall Street Inventory

### Marshall Street Inventory

Location (distance from Prentice St.)	Roadway Width	Shoulder	Objects	Pavement Markings
150' from Int. of Prentice St./ Marshall St.	22'	No curb	None	None
700'	21'	No curb	U.P. 1' off L	None
Site Driveway #1 800'	21'	No curb	U.P. 1' off L	None
Top of V.C.	21'	No curb	Embankment slope, U.P. w/ Tree off L	None
Site Driveway #2 1350'	22'	No curb	Trees	None
V.C. 250' from Driveway #2	21'	No curb	None	None
Soccer field/Emergency Access Driveway	21'	No curb	U.P. 1' off L	None
2500' from Soccer field	21'	No curb		None
2600' - 3500'		S Curv	e with Warning Signs	
3000' (House #255)	22'	No curb	None	None
3200'; Intersection with Great Meadow Road	22'	No curb	None	None
3300' Crest V.C.	22'	No curb	<200' S.D.	None
4500'	20'	1' Bit. Berm on R	U.P. 1' off L Tree 1' off R	None
4000' (House #355)	21'	No curb	U.P. 1' off L	None
4500'	19'	No curb	H.C. @ 2400' w/ steep slope R w/ no shoulder; V.C. @ 2600', U.P. 1' off L	None
4900' Hanlon St./Winston Rd.	20'	1' berm both sides	Poor S.D. from Winston Rd.	None
5500'	18'	No curb	Trees 2' off L and R Stonewall 3' L	None
5700'	N/A	No curb	Horizontal Curve	None
6000'	18'	No curb	Trees 1' L, 2' R Stonewall 2' L	None
Gorwin Drive	18'	Berm L South Corner	H.C. and V.C. to South limit sight distance	None
7000'	18'	No curb	Trees and U.P. 1' off L and R	None
7500' (House #695)	18'	No curb	U.P. 2' L and Trees 1' R	None
8000's Intersection with	22' N	T	NT	Faded